Module 2: Hydrologic Cycle

Concept & Average Global Water Balance

CWR 3540: Water Resources Engineering

FIU Department of Civil & Environmental Engineering

Professor Fuentes

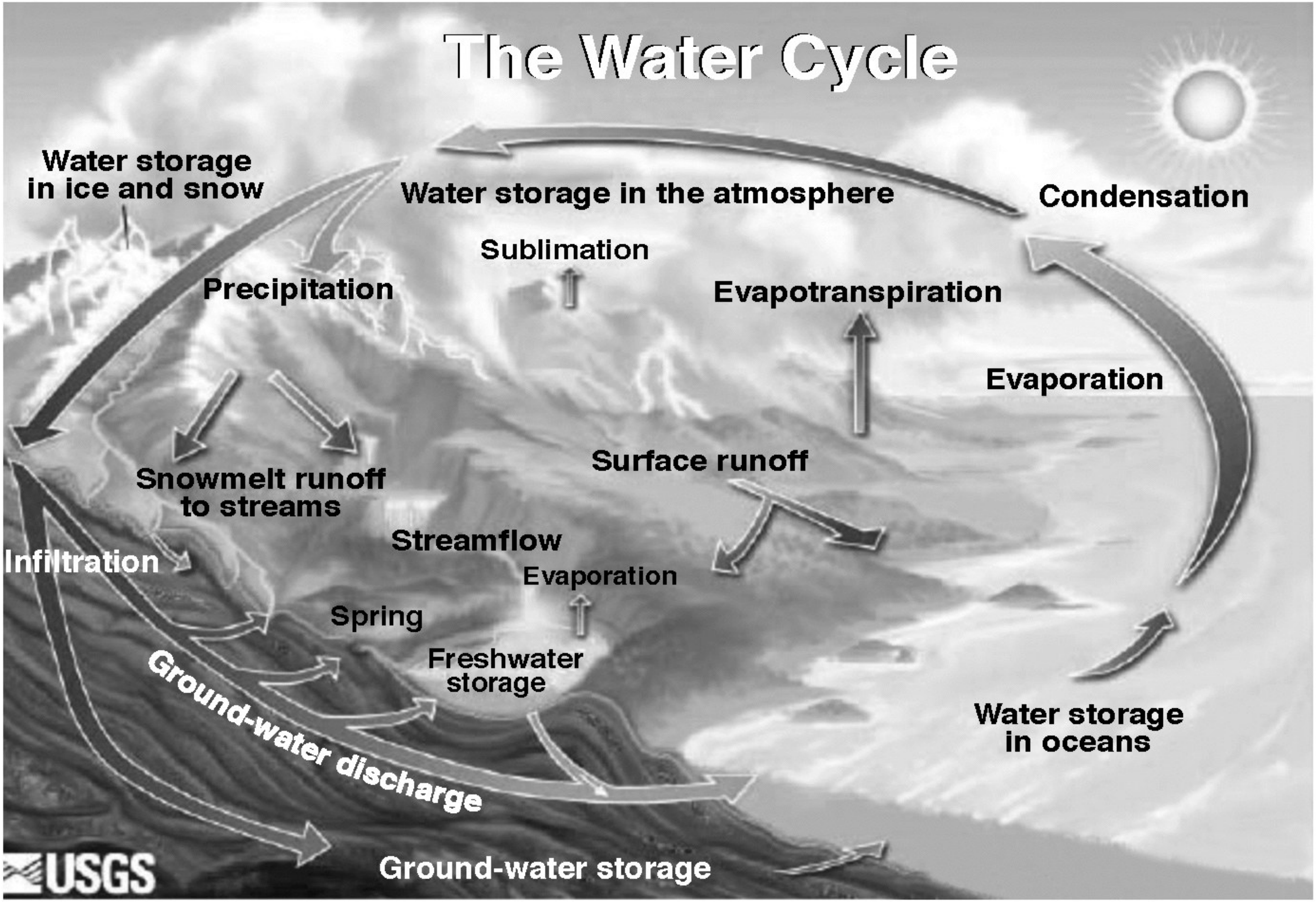

The Water Cycle
Water storage in ice and snow
Water storage in the atmosphere
Condensation
Sublimation
Precipitation
Evapotranspiration
Evaporation
Surface runoff
Snowmelt runoff to streams
Streamflow
Infiltration
Evaporation
Spring
Freshwater storage
Ground-water discharge
Water storage in oceans
USGS
Ground-water storage

Figure 1.1.1b

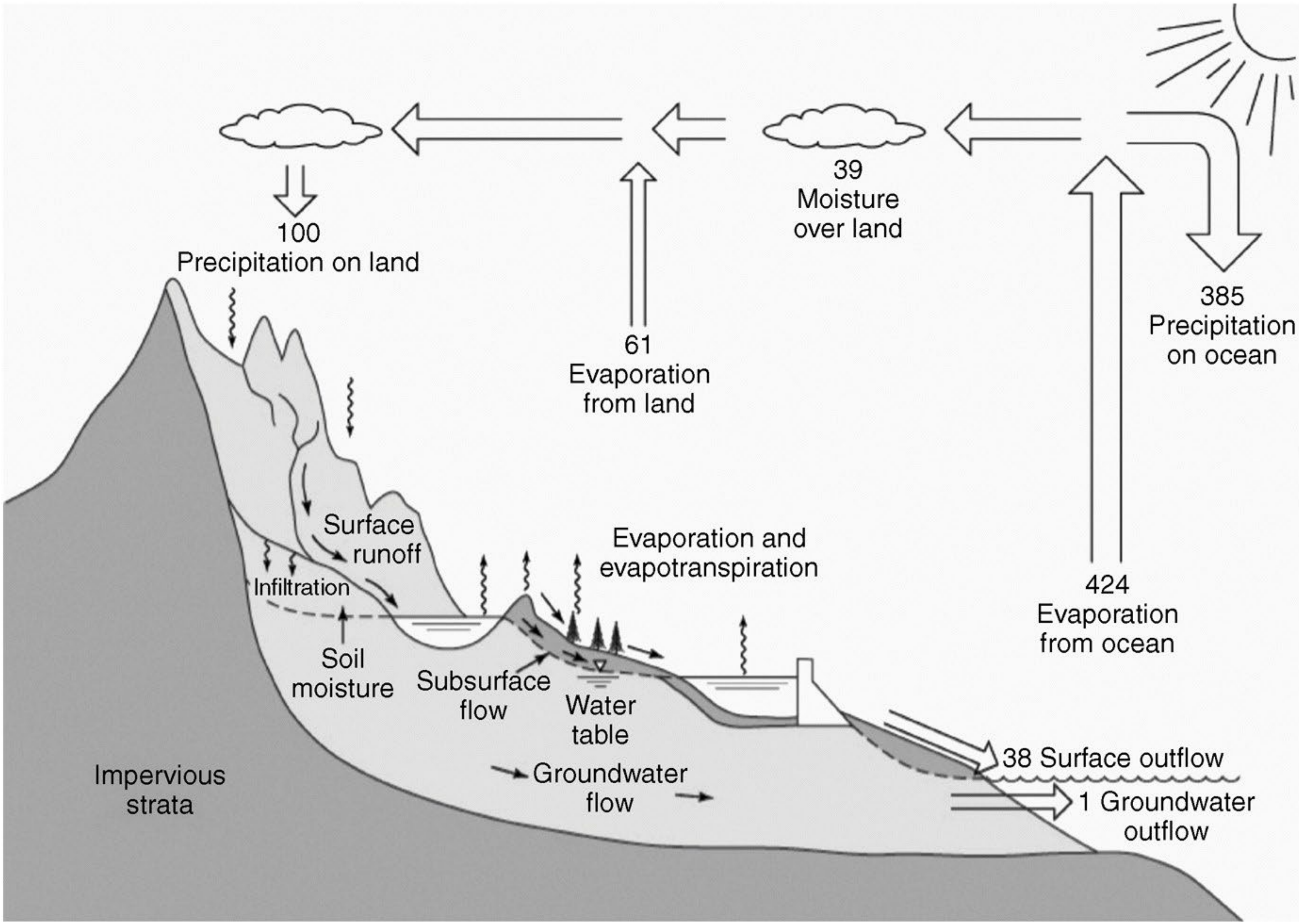


Figure 1.1.1a
McGraw Hill

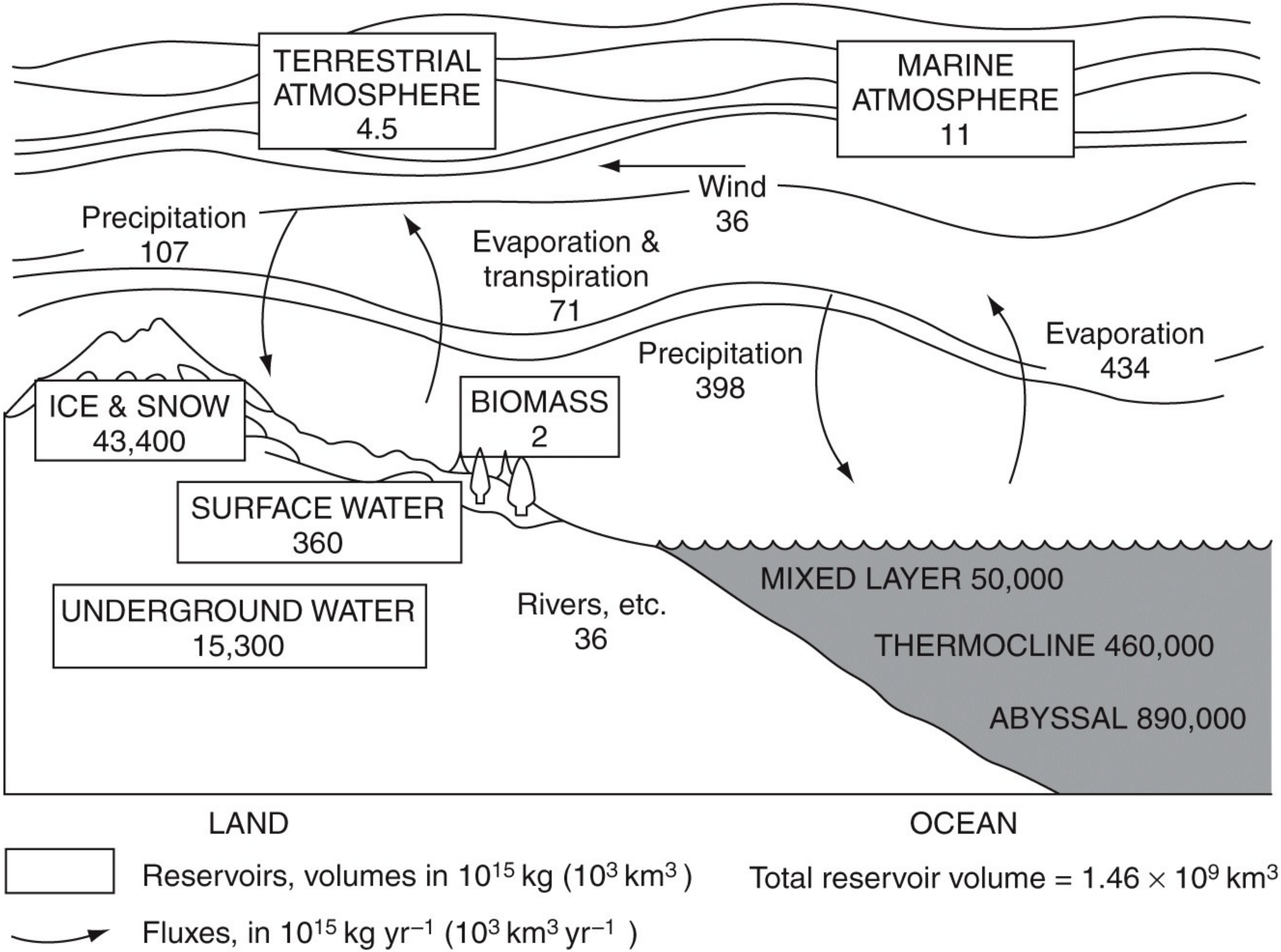


Figure 1.1.2
National Academic Press (National Academy of Sciences)

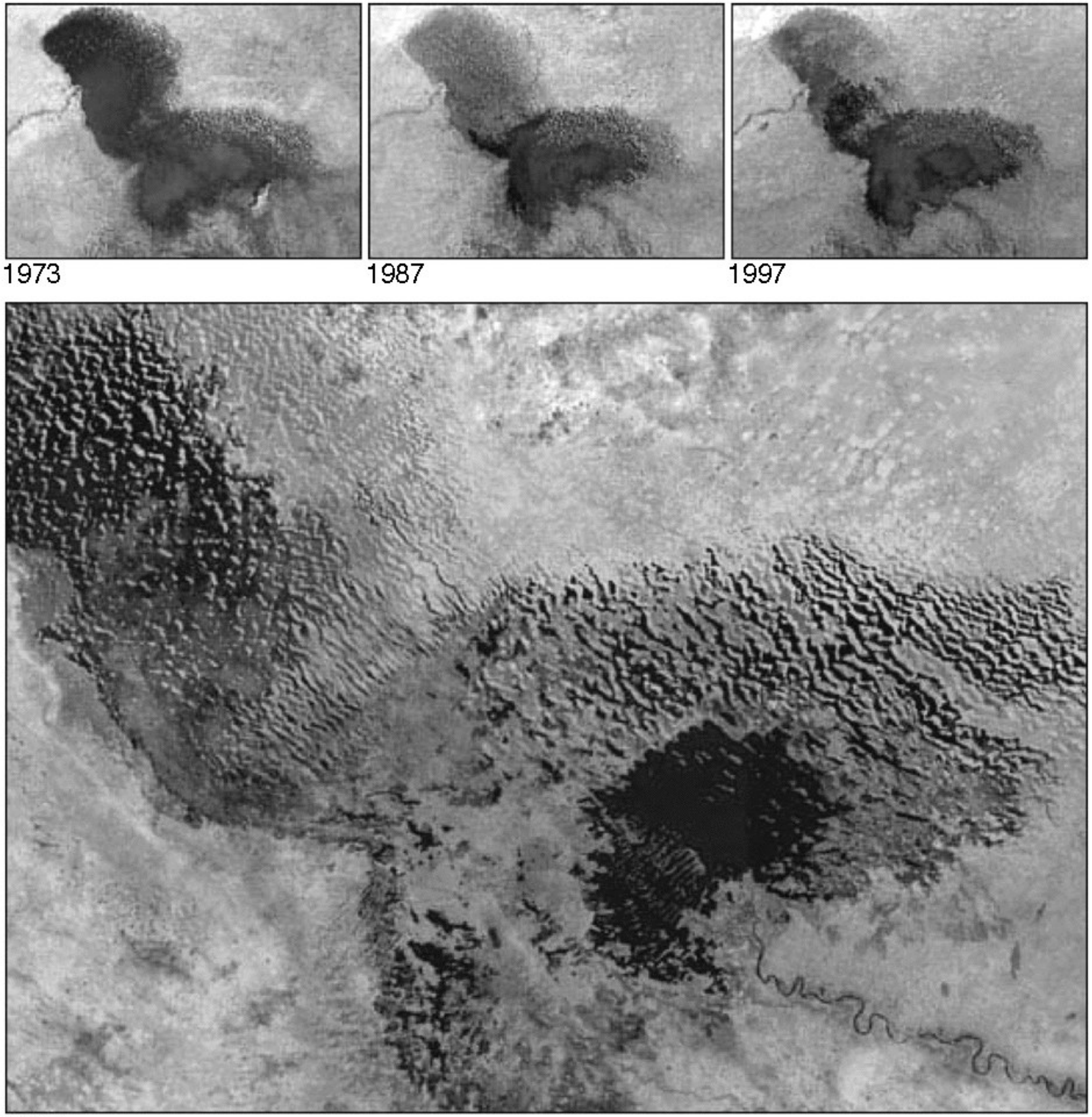


Figure 1.4.1
Images courtesy of NASA GSFC Scientific Visualization Studio and Landsat-7 Project

Module 2:
Hydrologic Cycle & Elements

Conservation of Mass Principle Handout:
(See in course website: Study Materials/Additional Handouts/Module 1)

CWR 3540: Water Resources Engineering

FIU Department of Civil & Environmental Engineering

Professor Fuentes

Control Volume Approach: Hydrosystems

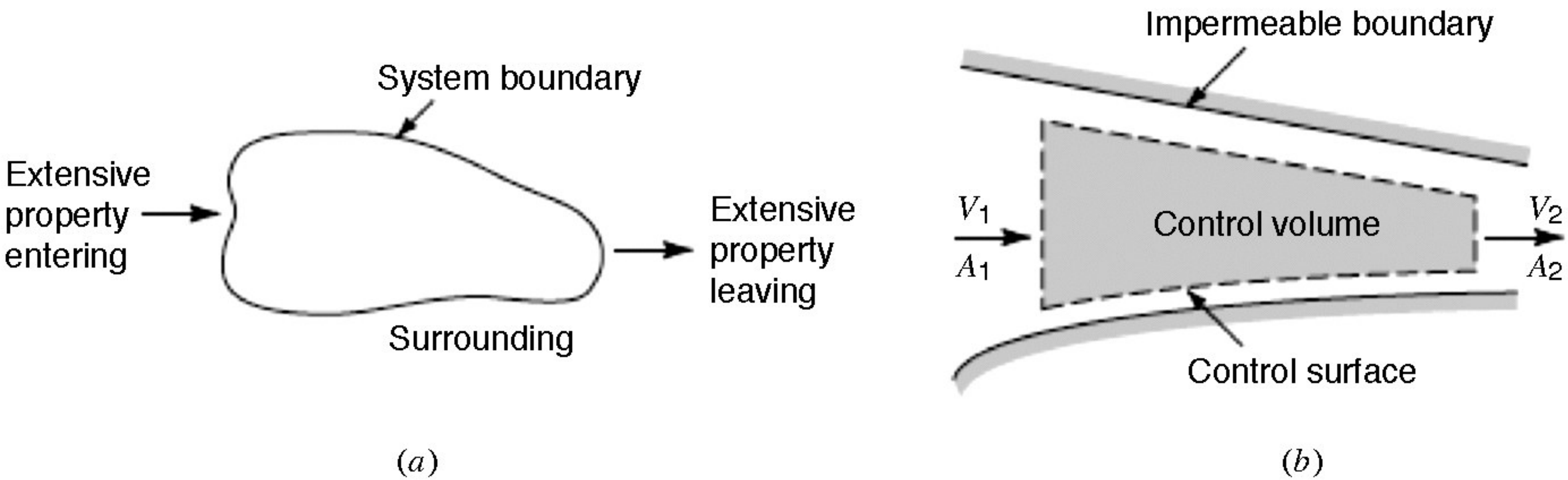


Figure 1.1.6

Drainage Basins, Watersheds, Catchments or Drainage Areas

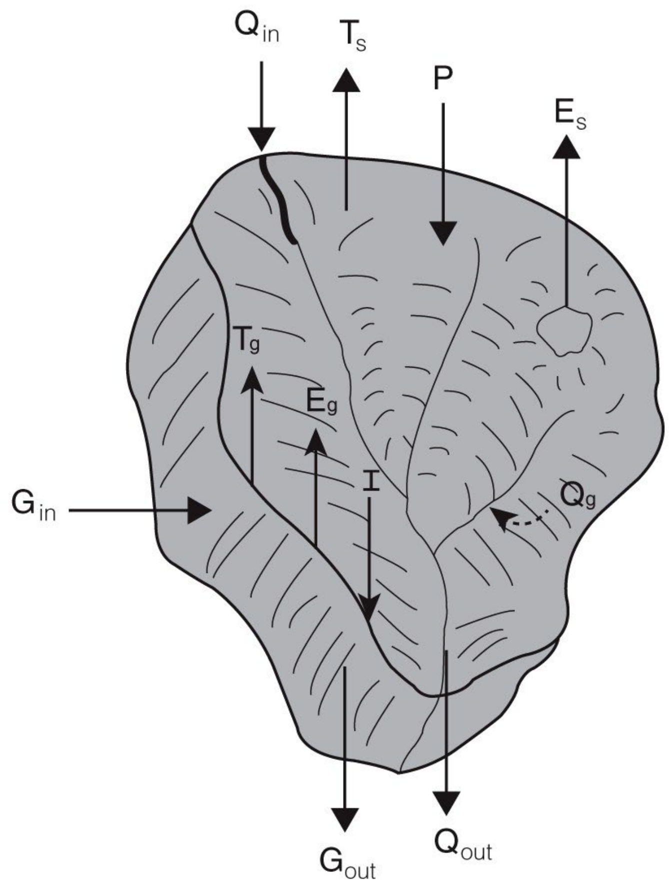


Figure 7.1.11

Sketch of a River Basin

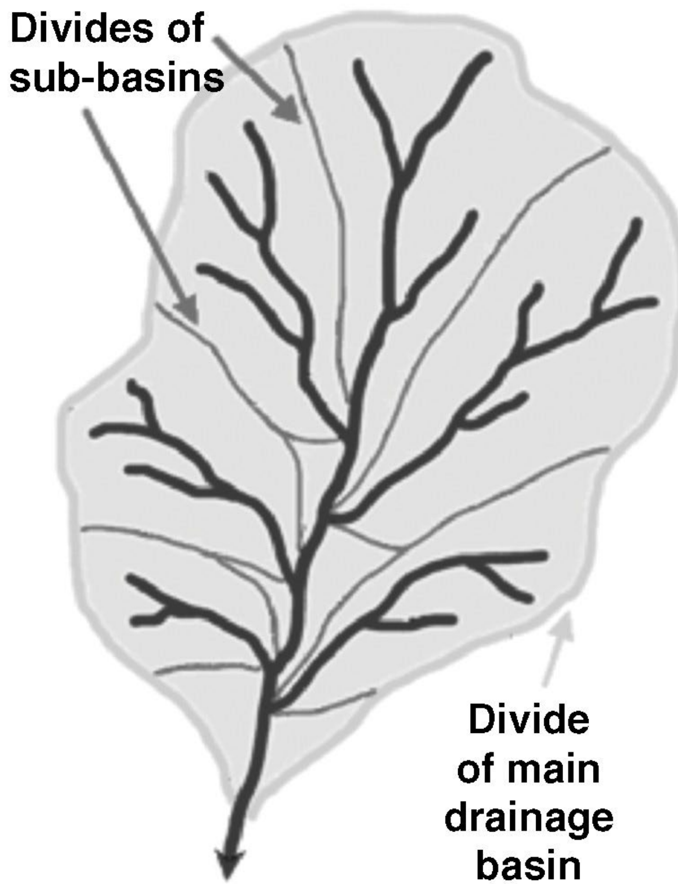
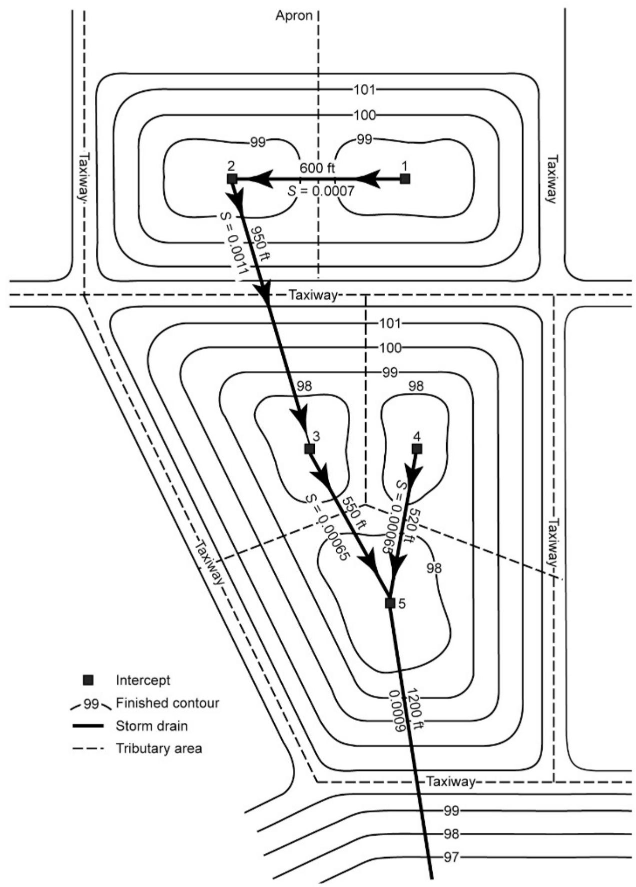


Figure 7.1.1

Airport Drainage Area: Example

Figure 17.10 Section of an airport.



Conservation of Mass Principle: Water Balance (or Budget) Equation(s)

- A general equation is Eq. 2.1:
  $P + Q_{SI} + Q_{GI} - E - Q_{SO} - Q_{GO} - \Delta s - n = 0$, for a ΔT (1 week, 1 year, etc.)
  When either volumes, flow rates or mean depths for the ΔT are used

- For water bodies, short-duration (Eq. 2.2):
  $P + Q_{in} - E - Q_0 - \Delta s = 0$
  If inflow and outflow terms in Eq. 2.1 are combined and n is "nil"

- For large basins for Long Duration (Eq. 2.3):
  $P - E - Q_0 = 0$
  If surface inflow, groundwater inflow and outflow terms in Eq. 2.1 an n are ignored or "nil"

- For direct runoff in a basin, over the ground surface, during a storm Eqs. 2.4,and2.5):
  $P - E - I - S_D - R = 0$ (Eq. 2.4)

  If evaporation, interception and depression storage are much smaller than infiltration, Eq. 2.4 becomes:
  $R = P - I$ (Eq. 2.5)

See Example 2.1

Discrepancy Term & Errors

Table 2.1 Percent Errors in Hydrologic Components by Commonly Used Methodologies

	Percent Error	
	Annual Estimate	Monthly Estimate
1. Precipitation		
Gage observation	2	2
Gage placement (height)	5	5
No windshield		20
Areal averaging	10	15
Gage density	13	20
2. Streamflow		
Current-meter measurement	5	5
Stage-discharge relationship	20	30
Channel bias	5	5
Regionalization of discharge	70	
3. Evaporation		
Energy budget	10	
Class A plan	10	10
Pan to lake coefficient	15	50
Areal averaging	15	15

Source: Based on Winter (1981).

Module 2: Hydrologic Cycle

Precipitation

CWR 3540: Water Resources Engineering

FIU Department of Civil & Environmental Engineering

Professor Fuentes

Forms of Precipitation

TABLE 4.1 Forms of Precipitation

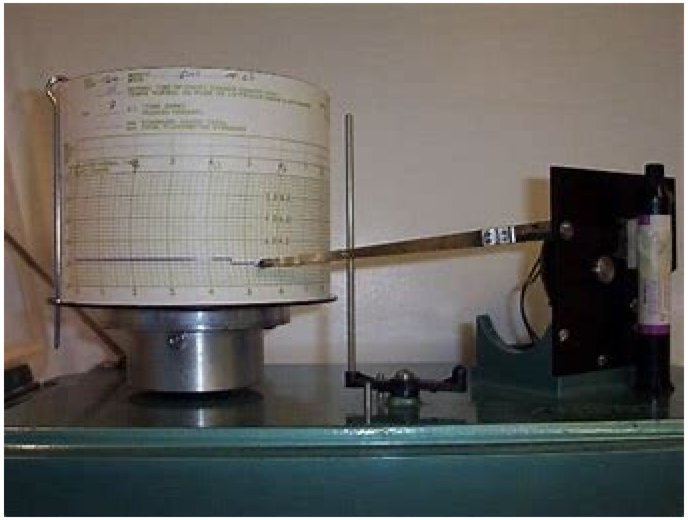
NAME	DESCRIPTION	SIZE
Drizzle	Water droplets, low intensity (1 mm hr^{-1})	0.1–0.5 mm
Rain	Water/drops Light: $\leq$2.5 mm hr^{-1} Moderate: 2.5–7.6 mm hr^{-1} Heavy: >7.6 mm hr^{-1}	>0.5 mm
Glaze	Ice coating, formed by freezing of rain or drizzle	Specific gravity ≈ 0.8
Rime	Opaque, granular ice deposit	Specific gravity ≈ 0.2–0.3
Snow	Ice crystals, hexagonal	Average specific gravity ≈ 0.1
Hail	Balls, irregular ice fragments; convective in nature	5 to over 125 mm; specific gravity ≈ 0.8
Ice pellets	Transparent, translucent ice	<5 mm

Source: Adapted from R. K. Linsley, Jr., M. A. Kohler, and J. L. H. Paulhus, *Hydrology for Engineers*, 3rd ed., McGraw-Hill.

Standard Rain Gage – Non-recording

Figure 12.1.6
Courtesy of the National Oceanic and Atmospheric Agency

Weighing Bucket Rain Gage - Recording



Tipping Bucket Rain Gage (Exterior)

Figure 12.1.8b
www.wikipedia.com

Example Rain Record and Hyetograph

TABLE 4.1 Hourly Precipitation Record for Washington, D.C., for September 1983

Hourly Precipitation (Water Equivalent in Inches)

Date	A.M. Hour Ending At												P.M. Hour Ending At												Date
	1	2	3	4	5	6	7	8	9	10	11	12	1	2	3	4	5	6	7	8	9	10	11	12	
1																									1
2																									2
3																									3
4																									4
5																									5
6																									6
7																									7
8																									8
9																									9
10																									10
11																									11
12															T	0.01			T	T	T		0.01	0.03	12
13	0.06	T						T	0.22	0.09	0.33	0.23	0.11	0.12	0.08	0.02	0.01	T							13
14																									14
15																									15
16																									16
17																	1								17
18																									18
19																									19
20																									20
21									T	T	T	T	0.01	0.06	T	0.21	0.17	0.40	0.05	0.02	0.07	0.03	T		21
22																									22
23												T	T												23
24																									24
25																									25
26																									26
27																									27
28																									28
29																									29
30				T	0.04	0.17	0.03	0.11	0.07	0.03	0.01	0.02	0:03	0.03	0.02	T	T								30

Note: T indicates trace amount.

MAXIMUM SHORT-DURATION PRECIPITATION

TIME PERIOD (MINUTES)	5	10	15	20	30	45	60	80	100	120	150	180
PRECIPITATION (INCHES)	0.12	0.18	0.21	0.26	0.35	0.40	0.45	0.50	0.58	0.63	0.75	0.81
ENDED: DATE	13	13	13	13	13	13	13	13	13	13	21	21
ENDED: TIME	1053	1058	1058	1100	1110	1115	1122	1142	1208	1222	1748	1818

THE PRECIPITATION AMOUNTS FOR THE INDICATED TIME INTERVALS MAY OCCUR AT ANY TIME DURING THE MONTH. THE TIME INDICATED IS THE ENDING TIME OF THE INTERVAL. DATE AND TIME ARE NOT ENTERED FOR TRACE AMOUNTS.

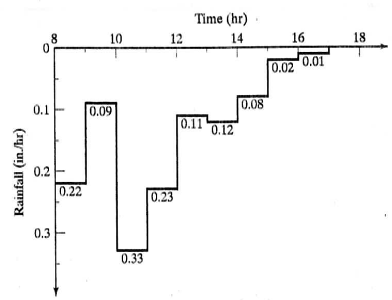


Figure 4.1 Hourly precipitation in Washington, D.C., on September 13, 1983.

SOURCE : McCuen, R.H., 3rd Edition, 2005
(Pearson, Prentice-Hall)

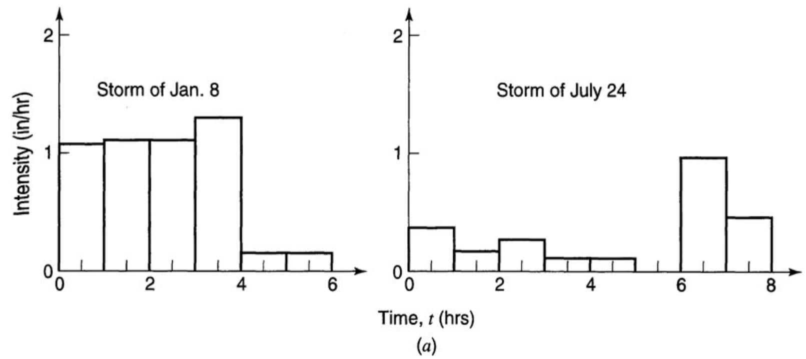
Rainfall Hyetographs

- *Hyetograph:*

  Plots of rainfall depth or intensity as function of time

Examples of Hyetographs:

Depth or intensity versus time
Cumulative (or "rainfall mass curve")



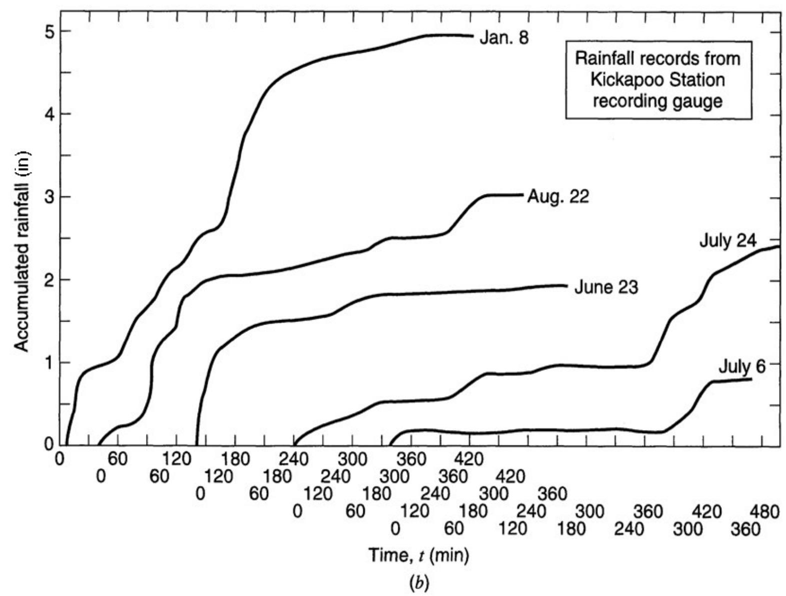


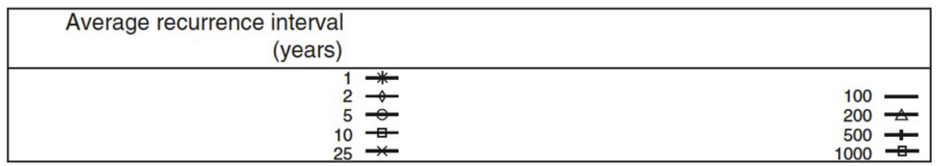
Figure 7.2.8

Return Period & Probability of Exceedance of Hydrologic Events

- $T = 1/P_{\geq}$

Precipitation Data Analysis (Mays, 2012)
(See Example 2.6 , Gupta 2017)

Mon Feb 02 21:45:19 2009



Precipitation Frequency Estimates (in)																		
ARI (years)	5 min	10 min	15 min	30 min	60 min	120 min	3 hr	6 hr	12 hr	24 hr	48 hr	4 day	7 day	10 day	20 day	30 day	45 day	60 day
1	0.38	0.60	0.73	0.97	1.18	1.38	1.48	1.77	2.04	2.39	2.74	3.15	3.66	4.13	5.55	6.96	8.73	10.4
2	0.46	0.71	0.88	1.17	1.44	1.68	1.81	2.16	2.48	2.91	3.32	3.77	4.36	4.89	6.56	8.20	10.24	12.2
5	0.55	0.85	1.05	1.43	1.80	2.12	2.30	2.78	3.17	3.72	4.21	4.67	5.28	5.88	7.73	9.49	11.65	13.9
10	0.62	0.96	1.18	1.63	2.08	2.47	2.68	3.30	3.75	4.39	4.93	5.40	6.02	6.69	8.64	10.46	12.69	15.2
25	0.71	1.09	1.34	1.90	2.46	2.94	3.21	4.05	4.58	5.37	5.99	6.46	7.06	7.84	9.88	11.70	13.95	16.7
50	0.78	1.19	1.47	2.10	2.77	3.33	3.64	4.70	5.28	6.20	6.88	7.33	7.90	8.79	10.85	12.62	14.87	17.9
100	0.85	1.28	1.60	2.31	3.08	3.73	4.09	5.40	6.05	7.10	7.83	8.27	8.78	9.78	11.82	13.49	15.71	18.9
200	0.93	1.39	1.72	2.52	3.41	4.15	4.57	6.17	6.89	8.10	8.88	9.27	9.70	10.84	12.80	14.34	16.50	19.9
500	1.03	1.52	1.90	2.81	3.88	4.75	5.24	7.31	8.13	9.58	10.41	10.77	11.06	12.33	14.12	15.42	17.47	21.1
1000	1.11	1.62	2.02	3.04	4.26	5.22	5.78	8.29	9.18	10.83	11.71	12.05	12.26	13.56	15.15	16.20	18.15	21.9

(a) Chicago, Ilinois 41.820 N 87.67 W 593 feet

Figure 7.2.15a part 2

Precipitation Data Analysis (Mays, 2012) (See Example 2.6, Gupta 2017)

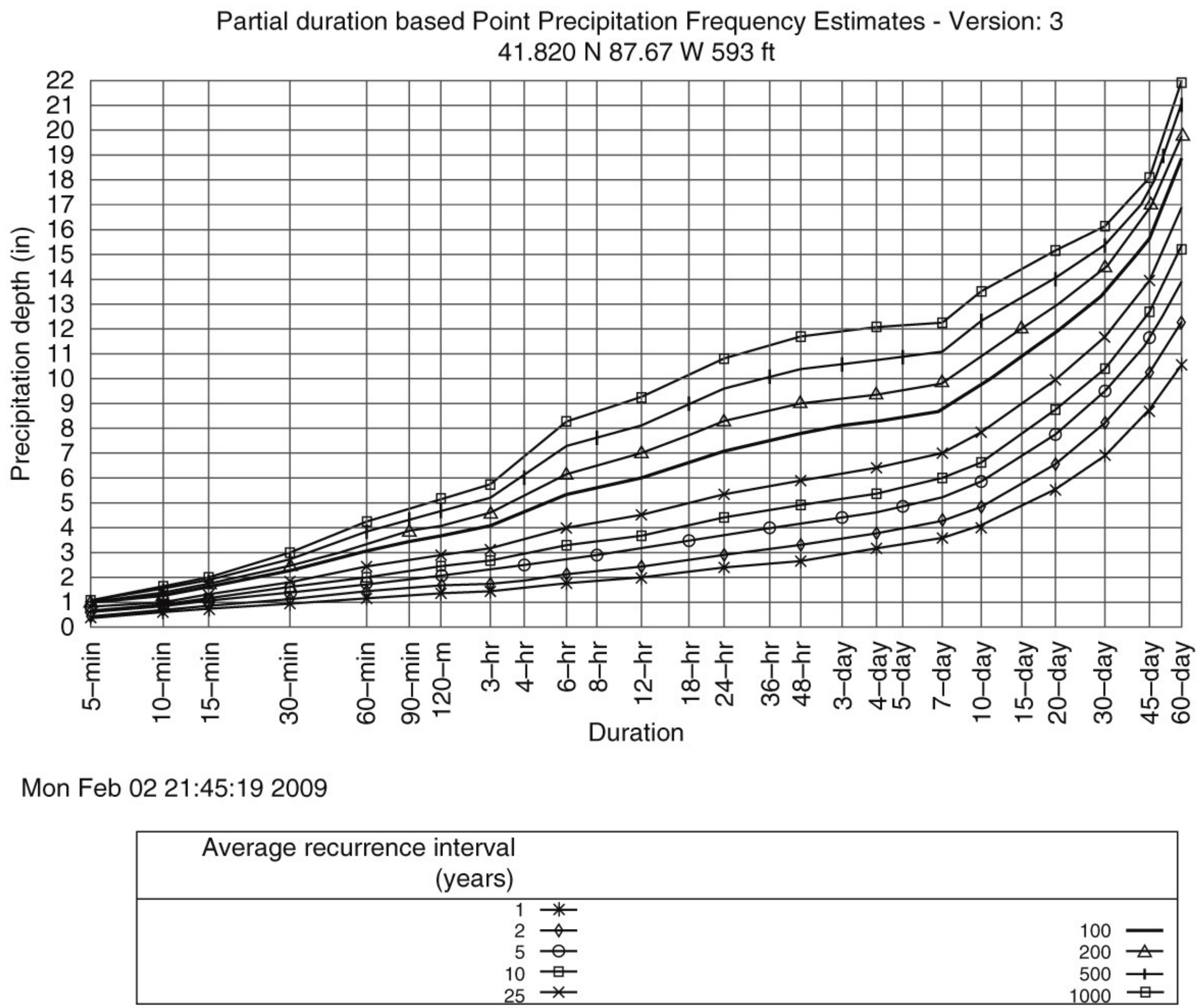


Figure 7.2.15a part 1

Example of IDFs (Example 2.6)

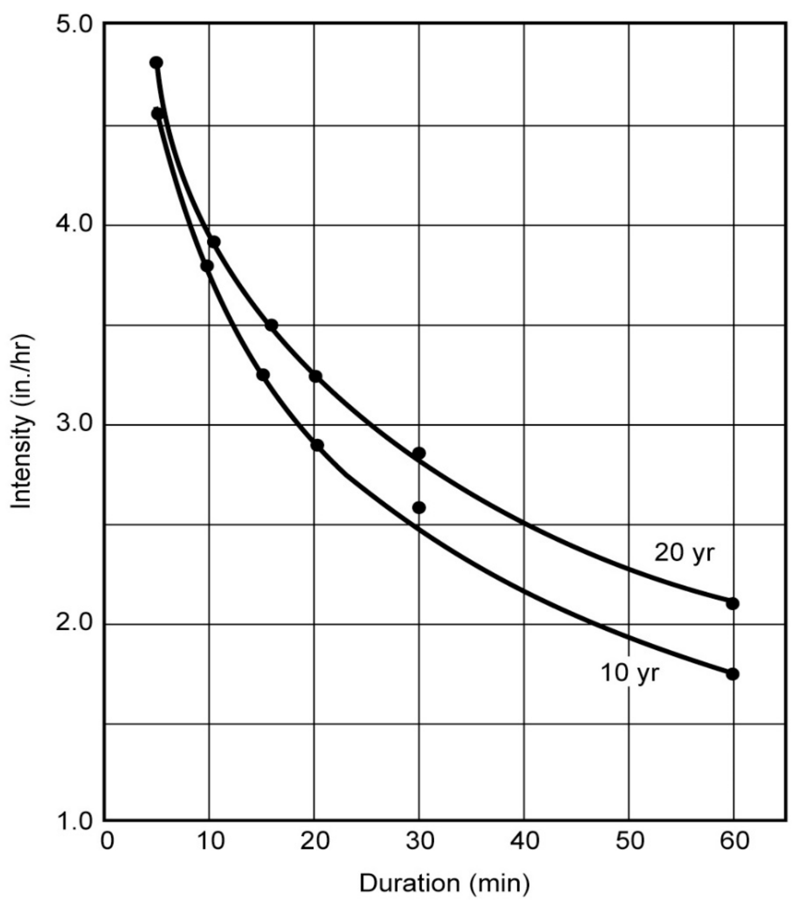
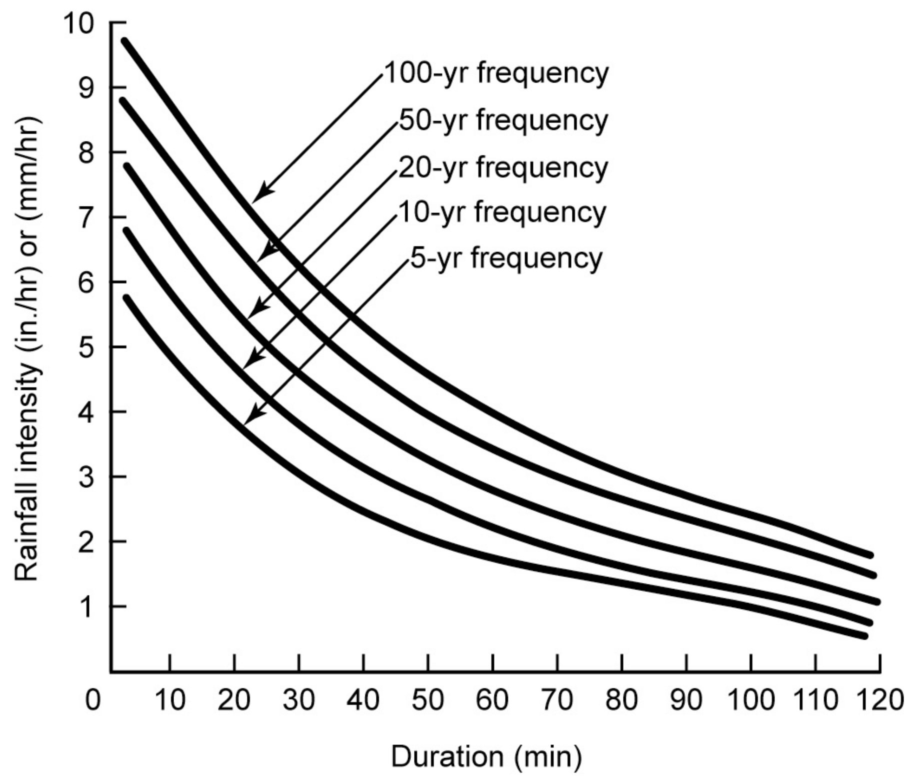


Figure 2.6 Intensity-duration-frequency curve.

Example of IDFs

Figure 16.6 Intensity-duration-frequency (IDF) curves for Bridgewater, CT.



Regression Analysis: IDFs

(A common, simple relationship)

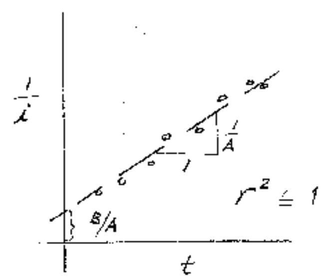
Linear Regression Analysis
for IDF-relation

$$i = \frac{A}{t + B} \quad \text{(Eq. 2.15, Gupta, 2017)}$$

Considering Equation 2.15 (Gupta, 2017), which applies to a specific "return period" in years, the following is an approach:

$$\frac{1}{i} = \frac{t + B}{A} \quad \therefore \quad \frac{1}{i} = \frac{1}{A} t + \frac{B}{A}$$

t	i
–	–
–	–
–	–



where r^2 = coefficient (or index) of determination
r = correlation coefficient

Intensity-Duration-Frequency Curve: "IDFs"

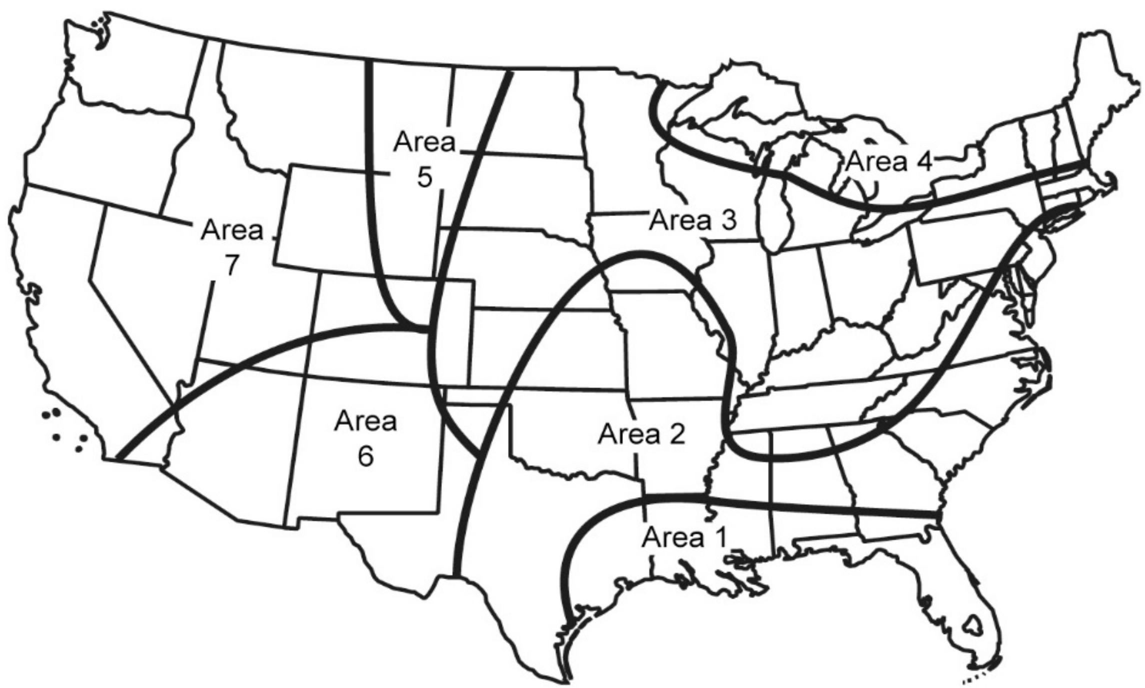
Example 2.6 (i.e., development of IDFs)

Equation 2.15 (e.g., empirical relationship)

NWS IDF Maps: Eq. 2.15 Regional Constants

IDFs Constants for USA Regions

Figure 2.7 Map of similar rainfall characteristics (from Steel and McGhee, 1979).



IDF General Relationships

A general equation for the IDF relations is the following:

$$i = \frac{c\,T^m}{t^e + f}$$

where

i = intensity

T = return period or average recurrence interval

t = storm duration

c, e, f = best-fit statistical constants

Solving Problems with Gages (Refer to Section 2.6)

- Two regular problems:
  - Breaks in records
  - Changes in recording conditions
- Solutions Using Neighboring Gage Sites:
  - Estimation of Missing Data
  - Eq. 2.14: $P_x/N^x = 1/n\ (P_1/N_1 + P_2/N_2 + \ldots + P_n/N_n)$
  - Checking Data Consistency: Double Mass-Analysis:
    - Plot of accumulated values at test gage site versus accumulated values at base stations (See Figure 2.2)

Estimating Missing Data from Gages

(See Equation 2.14 in textbook: $P_x/N_x = 1/n\ [\ P_1/N_1 + P_2/\ N_2 + \ldots.)$

("Additional example": file "1"
in folder Module2<Precipitation in course website)

As an example, consider the following data:

Gage	Annual P (in.)	Storm-Event P (in.)
A	42	2.6
B	41	3.1
C	39	2.3
X	41	?

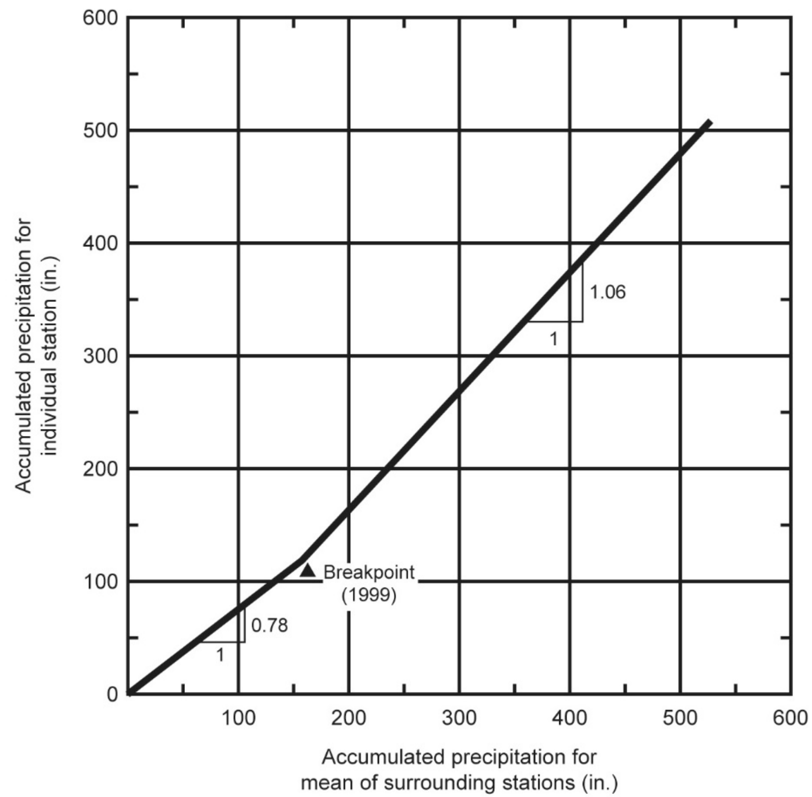
The storm-event catch at gage X is missing. Ten percent of the annual catch at gage X is 4.1 in., and the average annual catch at each of the three regional gages is within ± 4.1 in.; therefore, the station-average method can be used. The estimated catch at the gage with the missing storm-event total is

$$\hat{P} = \frac{1}{3}(2.6 + 3.1 + 2.3) = 2.67 \text{ in.} \tag{4.9}$$

Using this method requires knowledge of the average annual catch, even though this information is not used in computing the estimate, $\hat{P}$.

Checking Data Consistency: *Double-Mass Analysis (Example 2.3)*

Figure 2.3 Double-mass curve for Example 2.3.



Point to Areal Precipitation: Determination of Average

- Arithmetic or Station Average Method *(Example 2.4)*
- Weighted Average Methods
  - Thiessen Polygon Method *(Example 2.4)*
  - Isohyetal Method *(Example 2.5)*

Next Generation Radar (NEXRAD)

Radar Measurements

What is it?

Reflection of electromagnetic waves from a radar antenna by raindrops

Reflection strength =

f(number and size of drops)

Distance from radar site to the area =

f(time between pulse emission and receipt of echo)

Systems

NWS installed a network of 161 Doppler radar systems in the 1990's: NEXRAD for next generation radar. WSR88D was deployed jointly by USDOE, USDOD and USDOT.

Other systems are operated by FAA, among other agencies

Application

Estimates temporal intensities over large areas

Combined with rain gage data results in estimates between areas

Less accurate than gage measurements

NEXRAD – Example NWS

weather.gov

National Weather Service Enhanced Radar Mosaic

Southeast Sector

Go to: Standard Version   Local weather forecast by "City, St" City, St Go

Adjacent Sectors:

Sectors
Northern U.S.
Pac. Northwest Loop
Nrn. Rockies Loop
Upper Miss. Vly. Loop
Great Lakes Loop
Northeast Loop

Southern U.S.
Pac. Southwest Loop
Srn. Rockies Loop
Southern Plains Loop
Srn. Miss. Vly. Loop
Southeast Loop

U.S. Views
Reflectivity:
National Loop
Alaska Loop
Hawaii Loop
Guam Loop
Puerto Rico Loop

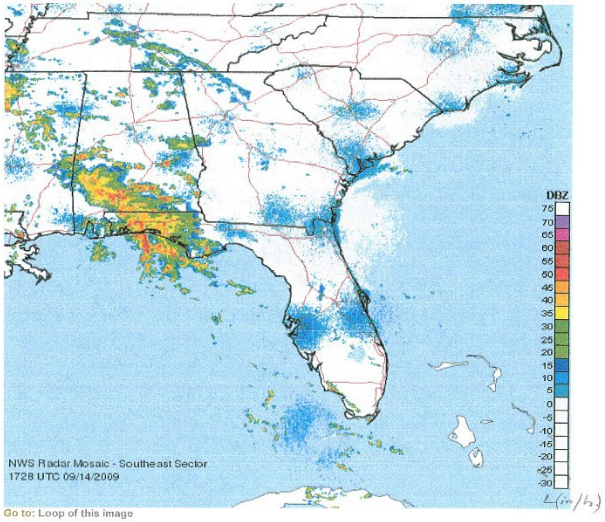
Radars by State
Go!

Additional Info:
Radar FAQ
Downloading Images
GIS Users
Doppler University
Color Blindness Tool
Credits

Base Reflectivity

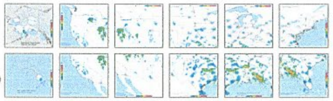
NWS is accepting comments on proposed combined warning and radar displays until October

Full resolution version (3400x1700 pixels - 220k)



Go to: Loop of this image

National Radar Mosaic Sectors (click image)



FIRSTGOV The U.S. Government's Official Web Portal

National Weather Service, NOAA
1325 East-West Highway
Silver Spring, MD 20910
Webmaster's E-mail: SR-SRH.Webmaster@noaa.gov
Page last modified: February 14, 2006

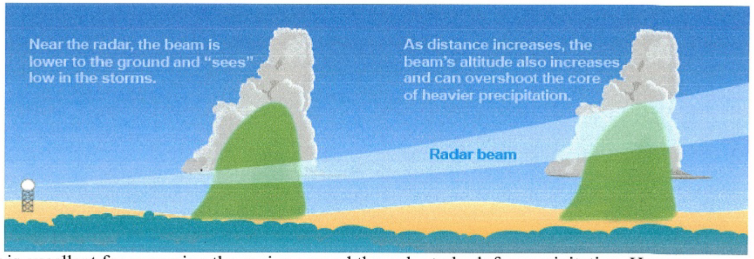
Disclaimer
Credits
Glossary

Privacy Policy
About Us
Career Opportunities

NATIONAL WEATHER SERVICE: *for Safety, for Work, for Fun* - FOR LIFE

NEXRAD – Example NWS (Cont.)

Base Reflectivity



Taken from the lowest (½°) elevation scan, base reflectivity is excellent for surveying the region around the radar to look for precipitation. However, remember the radar beam increases in elevation as distance increases from the radar. This is due, in part, to the elevation angle itself but is more because the earth's surface curves away from the beam.

This can lead to underestimating the strength and intensity of distant storms. For this reason, it is wise to always check the radar images from different locations to help provide the overall picture of the weather in any particular area.

This image (right) is a sample base reflectivity image from the Doppler radar in Frederick, OK. The radar is located in the center of the image. The colors represent the strength of returned energy to the radar expressed in values of decibels (dBZ). The color scale is located at the lower right of each image.

These dBZ values equate to approximate rainfall rates indicated in the table right.

These are hourly rainfall rates only and are not the actual amounts of rain a location receives. The total amount of rain received varies with intensity changes in a storm as well as the storm's motion over the ground.

Also, thunderstorms can contain hail which is often a good reflector of energy. Typically, a hailstone is coated with a thin layer of water as it travels through the thunderstorm cloud. This thin layer of water on the hailstone will cause a storm's reflectivity to be greater, leading to a higher dBZ and an over estimate the amount of rain received.

Value of 20 dBZ is typically the point at which light rain begins. The values of 60 to 65 dBZ is about the level where ¾" hail can occur. However, a value of 60 to 65 dBZ does not mean that severe weather is occurring at that location.

dBZ	Rain Rate (in/hr)
65	16+
60	8.00
55	4.00
52	2.50
47	1.25
41	0.50
36	0.25
30	0.10
20	Trace
< 20	No rain

Severe weather may be occurring with values less (or greater) than 60 to 65 dBZ due to...

- Hail that is totally frozen (without a thin layer of water in the surface). "Dry hail" is a very poor reflector of energy and can lead to an *underestimate* of a storm's intensity.
- Atmospheric conditions such a ducting. When ducting occurs, the radar beam is refracted into the ground (indicating stronger storms than what are actually occurring). However a worse case is

Module 2:
Hydrologic Cycle

Evaporation & Transpiration

CWR 3540: Water Resources Engineering

FIU Department of Civil & Environmental Engineering

Professor Fuentes

Evaporation & Transpiration

(“losses from runoff or groundwater)

- Evaporation: water vaporized into atmosphere from free water surface and land areas
- <u>Transpiration</u>: *water absorbed by plants and crops and eventually discharged into the atmosphere*

- *About 70% of the precipitation in the USA is returned to the atmosphere as either E, T or both, ET*

Evaporation Estimates (and Measurements) from Free-Water Bodies

- Comparative methods: Pan Evaporation and Atmometers
- Aerodynamic Method
- Energy Balance Method
- Combination Method (of the aerodynamic and energy methods)

Standard NWS Evaporation Pans (with Cup Anemometers)

Figure 12.1.12a
wikimedia commons

Class A Evaporation Pan

$(E_L = KE_p)$

Figure 12.1.12b
Courtesy of the National Weather Service

Evaporation Pans
(from a free-water body, e.g., a lake)

- Most common method: Uses the Standard National Weather Service Class A Pan (4 ft diameter x 10 in depth, wooden frame 12 in above ground
- Equation 3.1: $E_L = K \times E_p$

  E_L = evaporation from water body

  E_p = evaporation from the pan

  K = pan coefficient (0.6 to 0.8 range, and average of 0.7)

  Equation 3.2, Kohler and Parmele (1967) corrects K to a K' for monthly and daily evaporation losses, based on vapor pressure measurements:

$$E_L = K' \{(e_{sl} - e_z)/(e_{sp} - e_z)\} \times E_p$$

Refer to Example 3.1

Aerodynamic Method Sketch (Source: Mays, 2012)

It considers the transport of water vapor by the turbulence of the wind over a natural surface

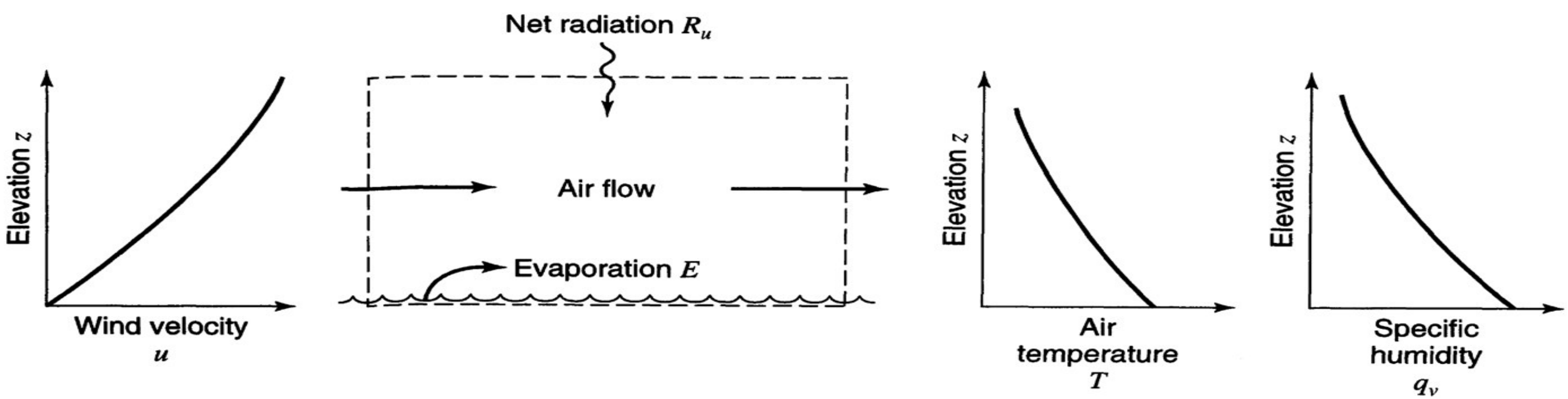


Figure 7.3.4

Aerodynamic Method

- Widely used for lakes and reservoirs
- Equations: 3.3, 3.4 and 3.5, with Table 3.1
  - $E_a = M(e_s – e_z)\ u_z$ (LT^{-1}), where
    - e_a = evaporation by aerodynamic method
    - M =mass transfer coefficient
    - e_s = saturation vapor pressure at water T
    - e_z = air vapor pressure at level z [= $RH(e_z^o)$]
    - RH = relative humidity
    - u_z = wind velocity at level Z
- See Example 3.2 and Additional Example in Module 2

Reference Height for Roughness

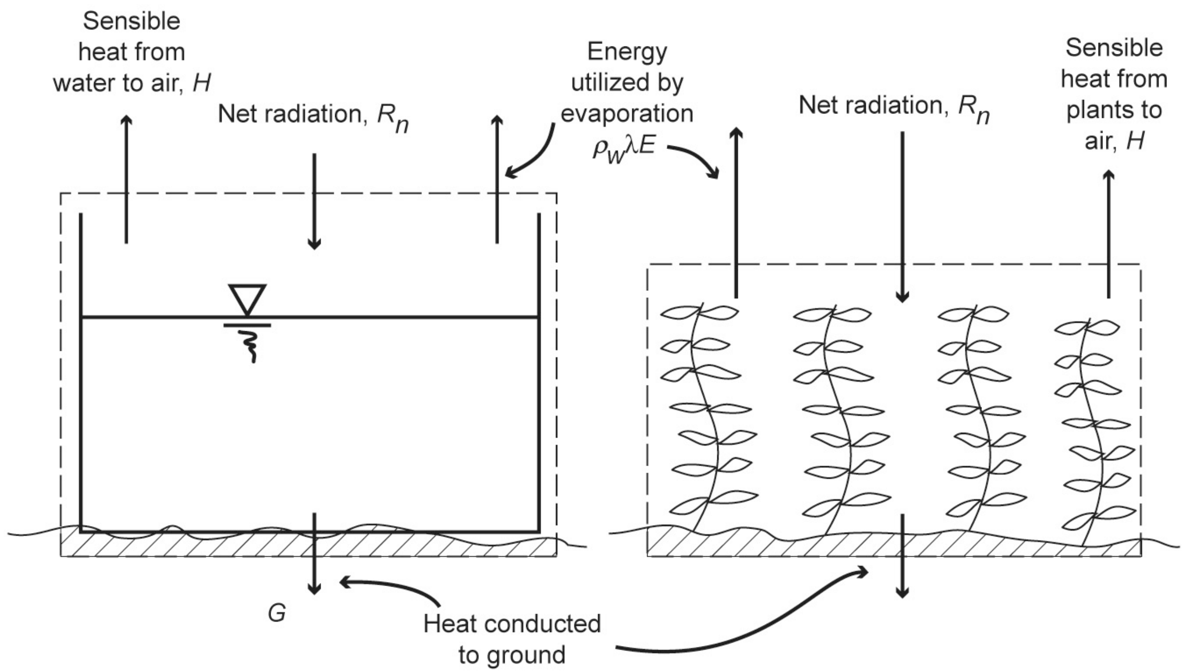
(Used when wind speed is measured at an elevation other than that for determination
(See Equation 3.5 or logarithmic law that accounts for surface roughness)

Table 3.1 Reference Height for Roughness

Roughness class	Roughness length, Z_0, m	Landscape
0	0.0002	Water surface
0.5	0.0024	Open terrain
1	0.03	Open agricultural area
1.5–2.5	0.055–0.2	Agricultural land with houses
3.0	0.4	Village, small town, forests
3.5	0.8	Larger cities
4.0	1.6	Very large cities

Energy Balance Method Sketch

Figure 3.1 Energy inflow and outflow from a water body and a cropped area.



Energy Method

- It is highly data intensive
- Equation 3.8, estimated as Eq. 3.9 or 3.10 plus Eq. 3.11supported by Equations 3.7, 3.8 and Table 3.1
  - $E_r = [R_n – G]/[\rho_w \lambda (1 + \beta)]$ $[LT^{-1}]$, where
    - Where, R_n = net radiant energy (= $S_n + R_b$), which are the net short-wave radiation and the net long-wave radiation, respectively
    - β, Bowen ratio, is calculated with Eq. 3.7
    - Other parameters are defined in Tables 3.2 and 3.3/ as function of temperature
- See Example 3.3

Combined Method (of Penman)

- Weighted estimate from both the aerodynamic and energy methods
- Equation 3.13
  - $E = \Delta/[\Delta + \gamma] \times E_r + \Delta/[\Delta + \gamma] \times E_a$, where
    - E_r = evaporation by the energy method
    - E_a = evaporation by the aerodynamic method
    - Δ = gradient of saturated water pressure, listed in Table 3.2 as function of temperature.
- See Example 3.4

C_p, e_s, Δ of e_s, γ, and λ

Table 3.2 Specific Heat, Saturated Vapor Pressure, Gradient, Psychrometric Constant, and Latent Heat of Vaporization at Standard Atmospheric Pressure

Temperature °C	Specific Heat c_p, kJ/kg °C	Saturated vapor pressure e_s, kPa	Gradient of saturated vapor pressure Δ, kPa °C^{-1}	Psychrometric constant γ, kPa °C^{-1}	Latent heat of vaporization λ, MJ/kg
0	4.218	0.611	0.044	0.0654	2.501
1	4.215	0.657	0.047	0.0655	2.499
2	4.211	0.706	0.051	0.0656	2.496
3	4.208	0.758	0.054	0.0656	2.494
4	4.205	0.814	0.057	0.0657	2.492
5	4.202	0.873	0.061	0.0658	2.489
6	4.200	0.935	0.065	0.0659	2.487
7	4.198	1.002	0.069	0.0659	2.484
8	4.196	1.073	0.073	0.0660	2.482
9	4.194	1.148	0.078	0.0660	2.480
10	4.192	1.228	0.082	0.0661	2.478
11	4.191	1.313	0.087	0.0661	2.475
12	4.190	1.403	0.093	0.0662	2.473
13	4.188	1.498	0.098	0.0663	2.470
14	4.187	1.599	0.104	0.0663	2.468
15	4.186	1.706	0.110	0.0664	2.466
16	4.185	1.819	0.116	0.0665	2.463
17	4.184	1.938	0.123	0.0665	2.461
18	4.184	2.065	0.130	0.0666	2.459
19	4.183	2.198	0.137	0.0666	2.456
20	4.182	2.337	0.145	0.0667	2.454
21	4.182	2.488	0.153	0.0668	2.451
22	4.181	2.645	0.161	0.0668	2.449
23	4.181	2.810	0.170	0.0669	2.447
24	4.180	2.985	0.179	0.0670	2.444
25	4.180	3.169	0.189	0.0670	2.442
26	4.180	3.363	0.199	0.0671	2.440
27	4.179	3.567	0.209	0.0672	2.437
28	4.179	3.781	0.220	0.0672	2.435
29	4.178	4.007	0.232	0.0673	2.433
30	4.178	4.243	0.243	0.0674	2.430
31	4.178	4.494	0.256	0.0674	2.428
32	4.178	4.756	0.269	0.0675	2.425
33	4.178	5.032	0.282	0.0676	2.423
34	4.178	5.321	0.296	0.0676	2.421
35	4.178	5.625	0.311	0.0677	2.418
36	4.178	5.943	0.326	0.0678	2.416
37	4.178	6.277	0.342	0.0678	2.414
38	4.178	6.627	0.358	0.0679	2.411
39	4.178	6.994	0.375	0.0680	2.409

ET from a Drainage basin

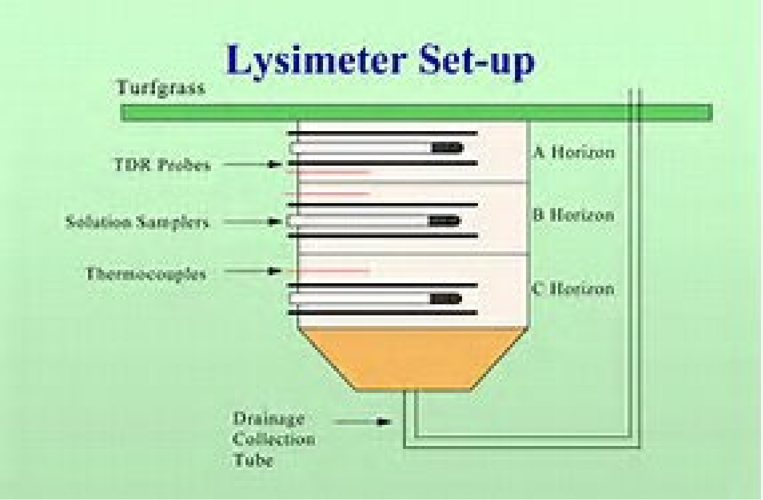
- Evapotranspirometers (i.e., experimental devices also referred to as *"lysimeters", if drainage from the soil is also accounted for)*
- Penman-Monteith ET Equation (after Eq. 3.13)
  - Equation 3.16 (p. 77), which defines two "resistances", one the aerodynamic resistance from a water surface by wind as water vapor transfer into the air above, r_a, and the resistance of water vapor to move from inside the plant to the air outside, r_s

Evapotranspirometers

(e.g., "lysimeters")

Example of Field Experiment

Example of System Drawing



Module 2: Hydrologic Cycle

Infiltration & Runoff

CWR 3540: Water Resources Engineering
FIU Department of Civil & Environmental Engineering
Professor Fuentes

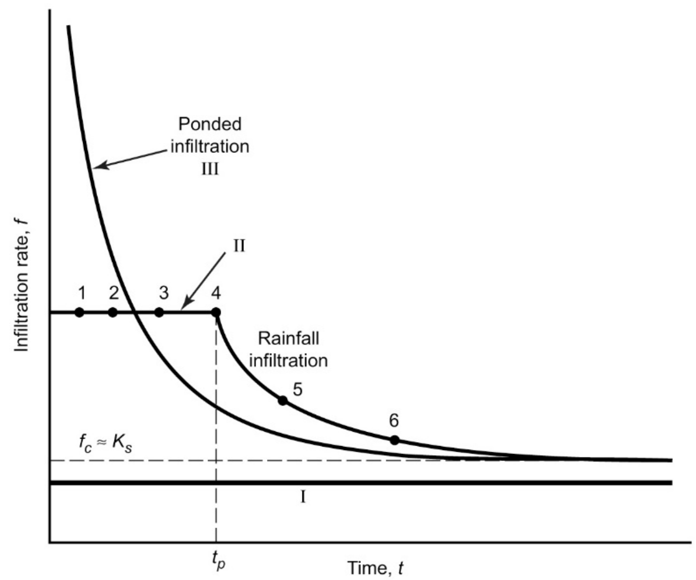
Double-ring Infiltrometer

Figure 12.1.15
www.wikipedia.com

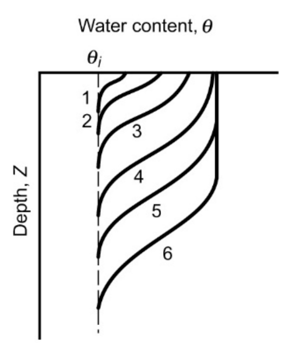
Infiltration Capacity Estimation

- Theory-based:
  - Infiltration Capacity and
  - Non-Linear Loss Rate Function
  - Green-Ampt Model (i.e., quite common)
- Empirical:
  - <u>Horton's Equation</u>
  - Holton's Equation & Others
- Methods for Direct Runoff that account for infiltration
  - <u>Infiltration-Index Method</u>
  - <u>NRCS (or "CN") Method</u>

Infiltration-Rainfall Behavior



(a)



(b)

Rainfall Intensity and Infiltration Rate

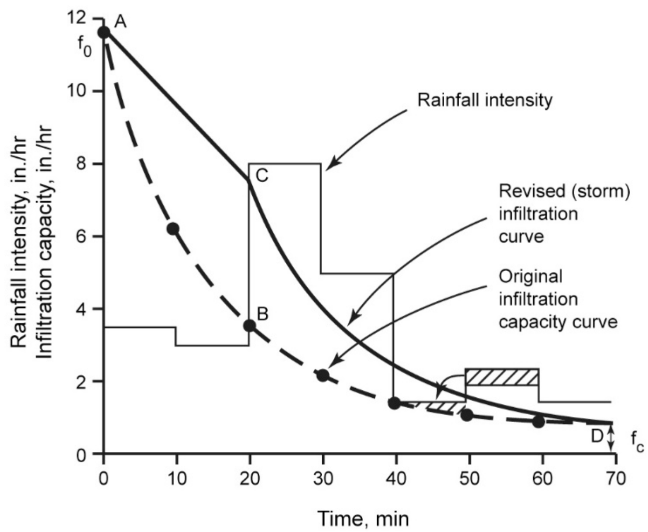


Figure 4.3 Rainfall intensity and infiltration capacity curves.

Horton's Infiltration Model

- Horton (1939):
  - $f_p = (f_o – f_c)\, e^{-kt} + f_c$ (Eq. 4.2, p. 96), where
  - f_o = initial infiltration rate
  - f_c = final constant infiltration rate, equal to *"apparent "soil" saturated hydraulic conductivity"*
  - k = factor representing the rate of decrease (also referred to as *"recession constant")*
  - *See handout and example in course website*

Horton's Model: Example 4.1

Table 4.1 Infiltration Capacity and Cumulated Infiltration

(1) Time min	(2) f_p in./hr	(3) Δt[a] min	(4) Average f_p[b] in./hr	(5) ΔF[c] in.	(6) F[d] in.
0	11.66				
		10	8.94	1.49	1.49
10	6.21				
		10	4.86	0.81	2.30
20	3.50				
		10	2.83	0.47	2.77
30	2.16				
		10	1.83	0.31	3.08
40	1.49				
		10	1.33	0.22	3.30
50	1.16				
		10	1.08	0.18	3.48
60	0.99				
		10	0.95	0.16	3.64
70	0.91				

[a] Successive difference col. 1

[b] Average of two successive values of col. 2

[c] col. 3 × col. 4 × $\left[\frac{1}{60}\frac{\text{hr}}{\text{min}}\right]$

[d] Cumulation of col. 5

Horton’s Model: Example 4.1

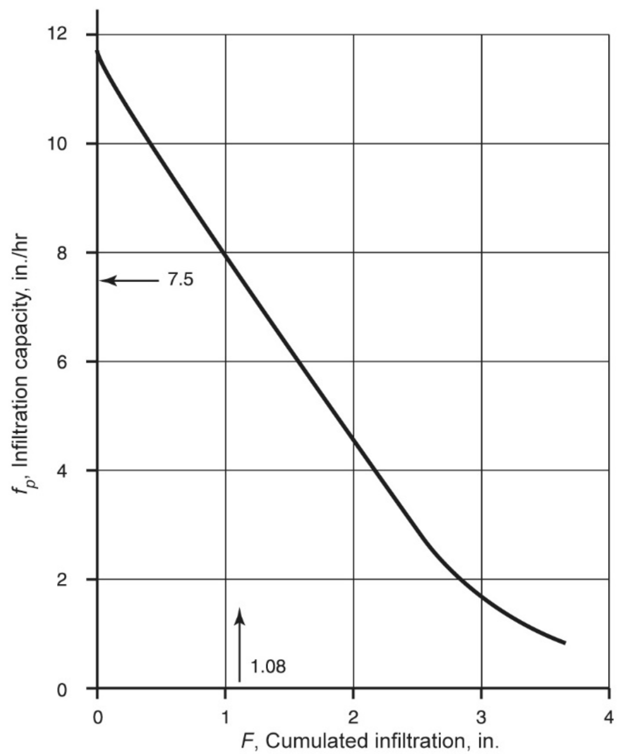


Figure 4.4 Cumulated infiltration curve.

Horton’s Model: Example 4.1

Table 4.2 Revised Infiltration Capacity for the Storm of Example 4.1

(1) t' min	(2) Time from beginning of storm t min	(3) f_p using t' in./hr
0	20	7.5
10	30	4.14
20	40	2.47
30	50	1.65
40	60	1.24
50	70	1.03

Horton's Model: Example 4.1

Table 4.3 Computations of Rainfall Excess by the Horton Method

(1) Time min	(2) Revised infiltration capacity f_p' in./hr	(3) Δt min	(4) Average f_p' in./hr	(5) Cumulated ΔF in.	(6) Rainfall intensity i in./hr	(7) $\Delta P = i\Delta t^{a}$ in.	(8) $RO^{b} = \Delta P - \Delta F$ in.
0							
10							
20	7.5						
		10	5.82	0.97	8.0	1.33	0.36
30	4.14						
		10	3.31	0.55	5.0	0.83	0.28
40	2.47						0
		10	2.06	0.34	1.5	0.25	~~−0.09~~
50	1.65						
		10	1.45	0.24	2.4	0.4	0.16
							(–).09[c]
60	1.24						= 0.07
		10	1.13	0.19	1.5	0.25	0.06
70	1.03						

Note: Computations of col. (3), (4) and (5) are similar to Table 4.1.

[a] Col. 6 × col. 3 × $\left[\frac{1}{60}\frac{\text{hr}}{\text{min}}\right]$

[b] Col. 7 – col. 5

[c] Negative value of the previous step.

Infiltration Index Method to Runoff

- See types of indices, including the simplest ϕ-Index option and its procedure in Section 4.5.
- Applications: Examples 4.6 and 4.7

Representation of the ϕ-Index

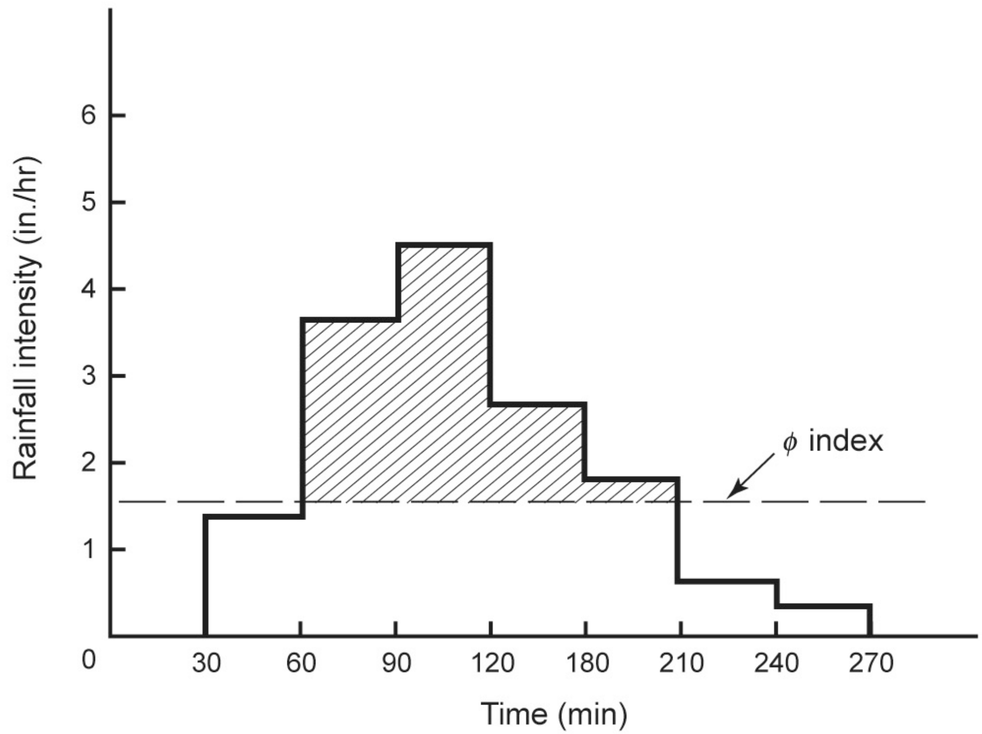


Figure 4.9
Representation of the ϕ index.

NRCS ("CN") Method for Direct Runoff

- $Q = (P - 0.2S)^2/(P + 0.8S)$ [L], where
  - Q = accumulated runoff depth over the drainage area
  - P= accumulated rainfall depth
  - S = potential maximum retention of water by soil
- $CN = 1000/(10 + S)$, where
  - CN = curve number (See Tables 4.10. 4.11, 4.12 and 4.13)

Hydrologic Soils Types

Table 4.10 Hydrologic Soil Groups

Group	Minimum Infiltration Rate (in./hr)	Texture[a]
A	0.3–0.45	Sand, loamy sand, or sandy loam
B	0.15–0.30	Silt loam or loam
C	0.05–0.15	Sandy clay loam
D	0–0.05	Clay loam, silty clay loam, sandy clay, silty clay, or clay

[a] Reproduced from U.S. Soil Conservation Service (1986).

Curve Numbers for AMC II

Table 4.11 Curve Numbers for Antecedent Moisture Condition II

Use	Cover Type	Treatment	Hydrologic Condition	Hydrologic soil group A	B	C	D
Urban	Fully developed						
	Open space (lawns, parks)		Poor (cover < 50%)	68	79	86	89
			Fair	49	69	79	84
			Good (grass cover > 75%)	39	61	74	80
	Impervious areas (paved parking, roofs, driveways, paved roads)			98	98	98	98
	Dirt roads			72	82	87	89
	Urban districts						
	Commercial and business			89	92	94	95
	Industrial			81	88	91	93
	Developing areas			77	86	91	94
Cultivated agriculture lands	Fallow	Bare soil	. . .	77	86	91	94
	Row crops	Straight row	Poor	72	81	88	91
		Straight row	Good	67	78	85	89
		Contoured	Poor	70	79	84	88
		Contoured	Good	65	75	82	86
		Contoured and terraced	Poor	66	74	80	82
		Contoured and terraced	Good	62	71	78	81
	Small grain	Straight row	Poor	65	76	84	88
		Straight row	Good	63	75	83	87
		Contoured	Poor	63	74	82	85
		Contoured	Good	61	73	81	84
		Contoured and terraced	Poor	61	72	79	82
		Contoured and terraced	Good	59	70	78	81
	Close-seeded	Straight row	Poor	66	77	85	89
	legumes	Straight row	Good	58	72	81	85
	or	Contoured	Poor	64	75	83	85
	rotation	Contoured	Good	55	69	78	83
	meadow	Contoured and terraced	Poor	63	73	80	83
		Contoured and terraced	Good	51	67	76	80

(continued)

Curve Numbers for AMC II

Table 4.11 Curve Numbers for Antecedent Moisture Condition II (Continued)

Use	Cover Type	Treatment	Hydrologic Condition	Hydrologic soil group A	B	C	D
Agriculture lands	Pasture		Poor	68	79	86	89
	or range		Fair	49	69	79	84
			Good	39	61	74	80
	Meadow			30	58	71	78
	Woods		Poor	45	66	77	83
			Fair	36	60	73	79
			Good	30	55	70	77
	Farmsteads (building, lanes, driveways)			59	74	82	86
Arid and semiarid rangelands	Herbaceous (mixture of grass,		Poor (< 30% ground cover)		80	87	93
	weeds, and low-growing brush)		Fair		71	81	89
			Good (> 70% cover)		62	74	85
	Oak–aspen (mountain brush mixture)		Poor		66	74	79
			Fair		48	57	63
			Good		30	41	48
	Pinyon–juniper		Poor		75	85	89
			Good		41	61	71
	Sagebrush with grass understory		Poor		67	80	85
			Good		35	47	55
	Desert shrub		Poor	63	77	85	88
			Fair	55	72	81	86
			Good	49	68	79	84

Source: Condensed from U.S. Soil Conservation Service (1986).

Curve Numbers for AMC II

Table 4.12 Antecedent Moisture Condition

Category	Condition
I	Dry soil but not to the wilting point
II	Average conditions
III	Saturated soils; heavy rainfall or light rainfall with low temperatures has occurred in the last 5 days

Curve Numbers for AMC I and II As Function of AMC II

Table 4.13 Cross-Linking of Curve Numbers for Various Antecedent Moisture Conditions

Curve Number for Condition	Corresponding Curve Number for Condition	
II	I	III
100	100	100
95	87	99
90	78	98
85	70	97
80	63	94
75	57	91
65	45	83
60	40	79
55	35	75
50	31	70
45	27	65
40	23	60
35	19	55
30	15	50
25	12	45
20	9	39
15	7	33
10	4	26
5	2	17
0	0	0

Source: U.S. Soil Conservation Service (1972).

Runoff Depth As Function of CN

Table 4.14 Runoff Depth for Selected CNs and Rainfall Amounts[a]

	Runoff Depth (in.) for Curve Number of:												
Rainfall	40	45	50	55	60	65	70	75	80	85	90	95	98
0.5	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.01	0.06	0.17	0.32
1.0	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.03	0.08	0.17	0.32	0.56	0.79
1.2	0.00	0.00	0.00	0.00	0.00	0.00	0.03	0.07	0.15	0.27	0.46	0.74	0.99
1.4	0.00	0.00	0.00	0.00	0.00	0.02	0.06	0.13	0.24	0.39	0.61	0.92	1.18
1.6	0.00	0.00	0.00	0.00	0.01	0.05	0.11	0.20	0.34	0.52	0.76	1.11	1.38
1.8	0.00	0.00	0.00	0.00	0.03	0.09	0.17	0.29	0.44	0.65	0.93	1.29	1.58
2.0	0.00	0.00	0.00	0.02	0.06	0.14	0.24	0.38	0.56	0.80	1.09	1.48	1.77
2.5	0.00	0.00	0.02	0.08	0.17	0.30	0.46	0.65	0.89	1.18	1.53	1.96	2.27
3.0	0.00	0.02	0.09	0.19	0.33	0.51	0.71	0.96	1.25	1.59	1.98	2.45	2.77
3.5	0.02	0.08	0.20	0.35	0.53	0.75	1.01	1.30	1.64	2.02	2.45	2.94	3.27
4.0	0.06	0.18	0.33	0.53	0.76	1.03	1.33	1.67	2.04	2.46	2.92	3.43	3.77
4.5	0.14	0.30	0.50	0.74	1.02	1.33	1.67	2.05	2.46	2.91	3.40	3.92	4.26
5.0	0.24	0.44	0.69	0.98	1.30	1.65	2.04	2.45	2.89	3.37	3.88	4.42	4.76
6.0	0.50	0.80	1.14	1.52	1.92	2.35	2.81	3.28	3.78	4.30	4.85	5.41	5.76
7.0	0.84	1.24	1.68	2.12	2.60	3.10	3.62	4.15	4.69	5.25	5.82	6.41	6.76
8.0	1.25	1.74	2.25	2.78	3.33	3.89	4.46	5.04	5.63	6.21	6.81	7.40	7.76
9.0	1.71	2.29	2.88	3.49	4.10	4.72	5.33	5.95	6.57	7.18	7.79	8.40	8.76
10.0	2.23	2.89	3.56	4.23	4.90	5.56	6.22	6.88	7.52	8.16	8.78	9.40	9.76
11.0	2.78	3.52	4.26	5.00	5.72	6.43	7.13	7.81	8.48	9.13	9.77	10.39	10.76
12.0	3.38	4.19	5.00	5.79	6.56	7.32	8.05	8.76	9.45	10.11	10.76	11.39	11.76
13.0	4.00	4.89	5.76	6.61	7.42	8.21	8.98	9.71	10.42	11.10	11.76	12.39	12.76
14.0	4.65	5.62	6.55	7.44	8.30	9.12	9.91	10.67	11.39	12.08	12.75	13.39	13.76
15.0	5.33	6.36	7.35	8.29	9.19	10.04	10.85	11.63	12.37	13.07	13.74	14.39	14.76

[a] Interpolate the values shown to obtain runoff depths for CNs or rainfall amounts not shown.
Source: U.S. Soil Conservation Service (1986).

[Florida air quality check: Thank Canada fire smoke for hazy Florida (news-journalonline.com)]

Module 2: Hydrologic Cycle

Streamflow

CWR 3540: Water Resources Engineering

FIU Department of Civil & Environmental Engineering

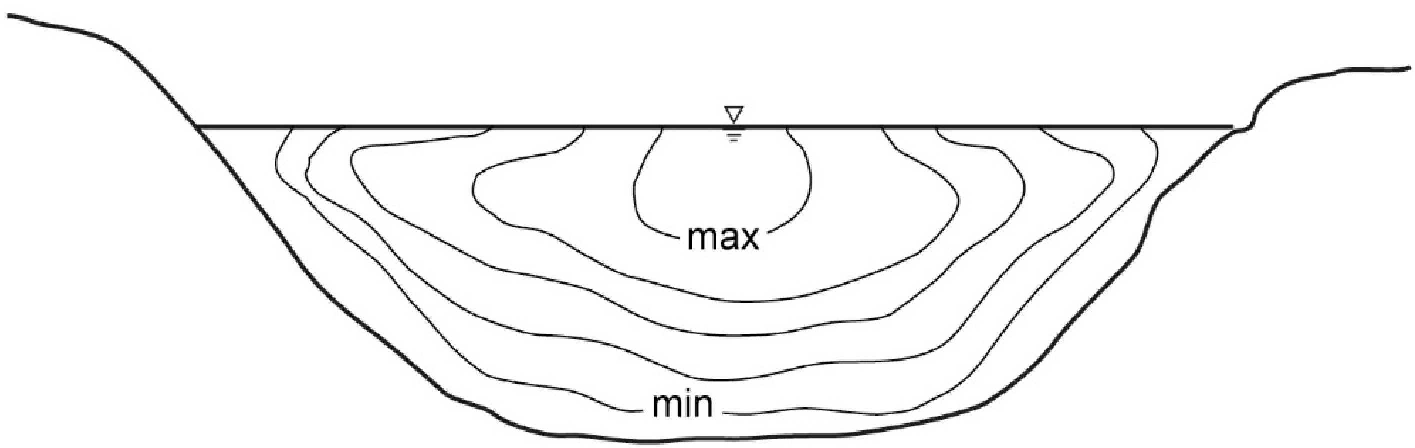
Professor Fuentes

Streams, Rivers & Canals




Stream Channel Velocity-Distribution

Figure 8.5 Typical velocity distribution in a stream channel.



Determination of Streamflow
(*or Discharge*)

- *Streamflow* quantity, space distribution and time variability are required to plan and design any surface water supply or hydraulic structure.

Measurement & Estimation

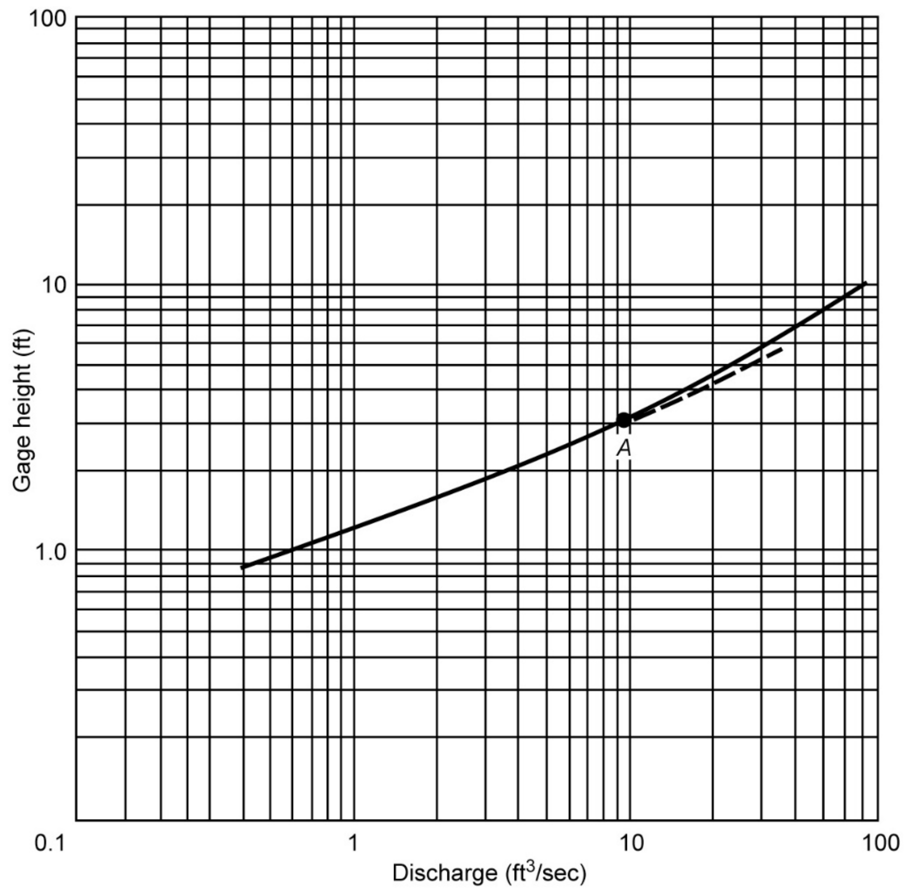
- Measurement of Streamflow
  - Direct measurement or stream gaging
  - Through hydraulic devices (for small stream and open channels, for instance, weirs, flumes, etc.)
  - Indirect measurement of peak flows
- Estimation of Streamflow
  - Application of precipitation data
  - Extension of gage-sites data
  - Generation of synthetic flows
  - Use of generalized data, charts, tables and empirical approaches

Stream Gaging

- Stream gaging or hydrometry measures the water stage (level or depth) and discharge at a gaging station to collect a continuous record
- A number of gaging stations in a watershed, basin or drainage area form a hydrologic network
- The US Geological Survey operates a nationwide stream-gaging network (+7,600)

Stage-Discharge Relationship: ("Rating Curve")

Figure 8.23 Log-log plot of stage and discharge data for Example 8.11.

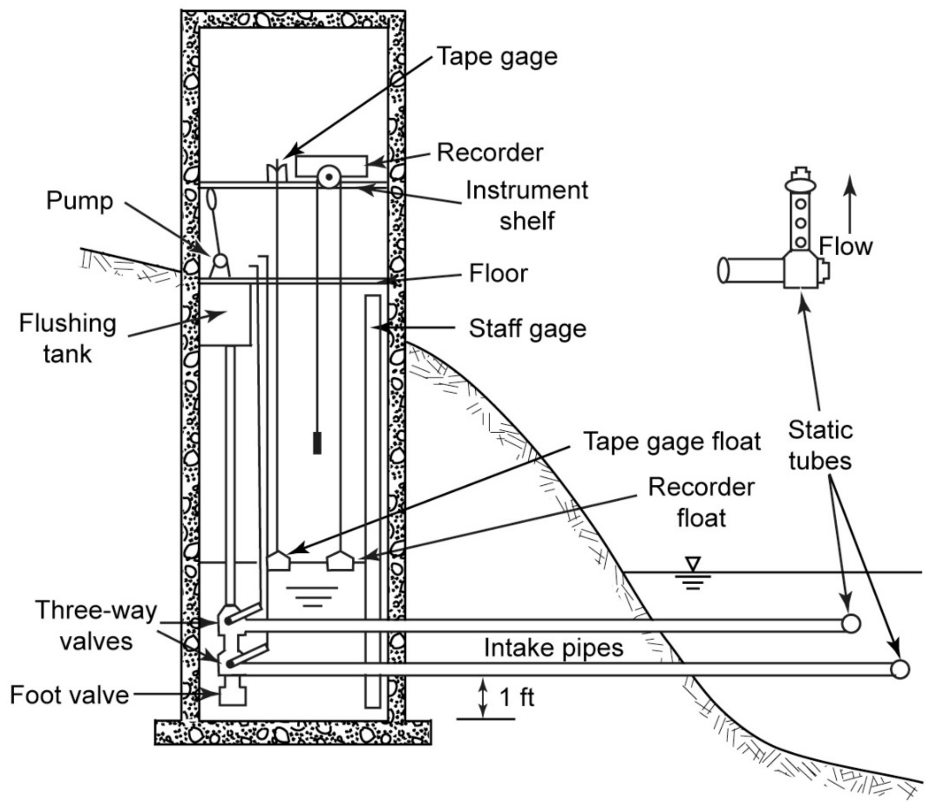


Stage Measurement

- Stilling Well with Float Sensor (Fig. 8.1)
- Pressure System with Bubble-Gage Sensor (Fig. 8.2) (e.g., nitrogen)
- Radar Stage Measurement (Fig. 8.3) (e.g., sensor-emitted electromagnetic waves echoed back to the sensor)

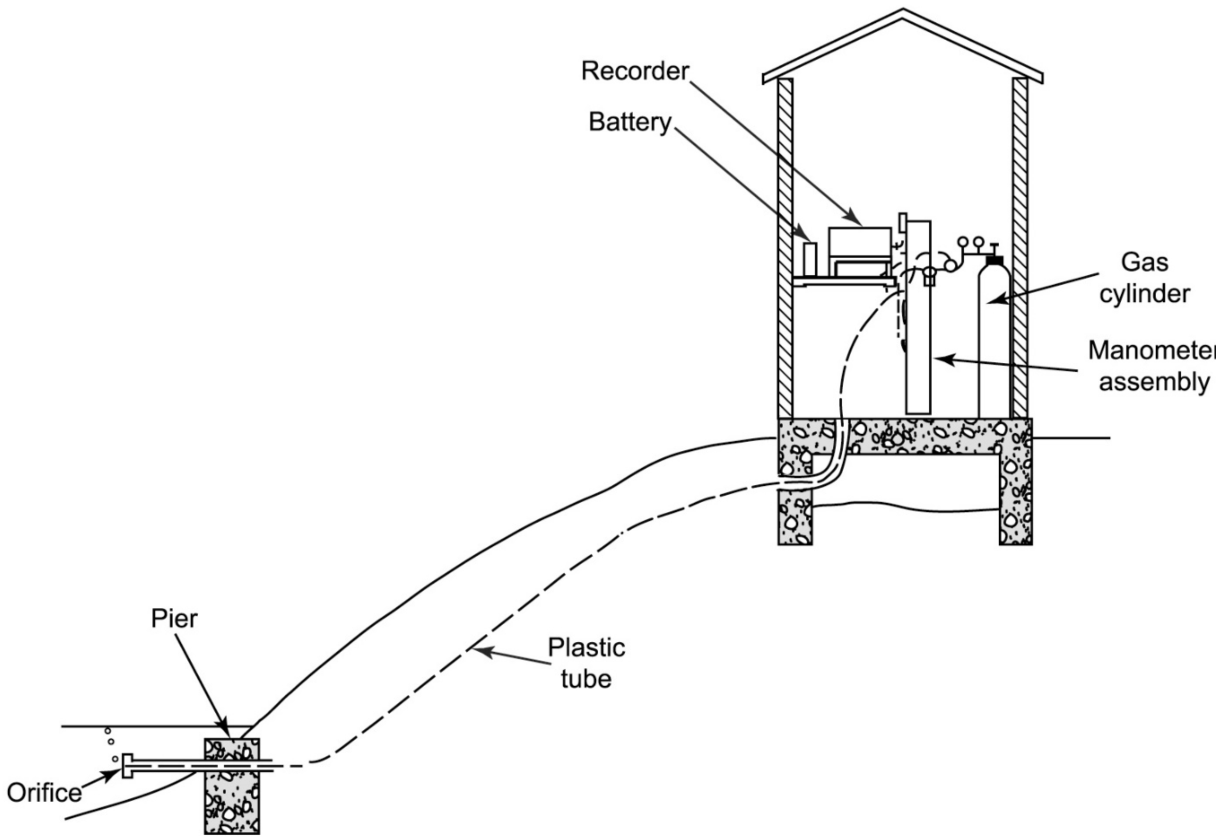
Stage Measurement

Figure 8.1 Stilling well for a float-type recorder (from Herschy, 1985a).



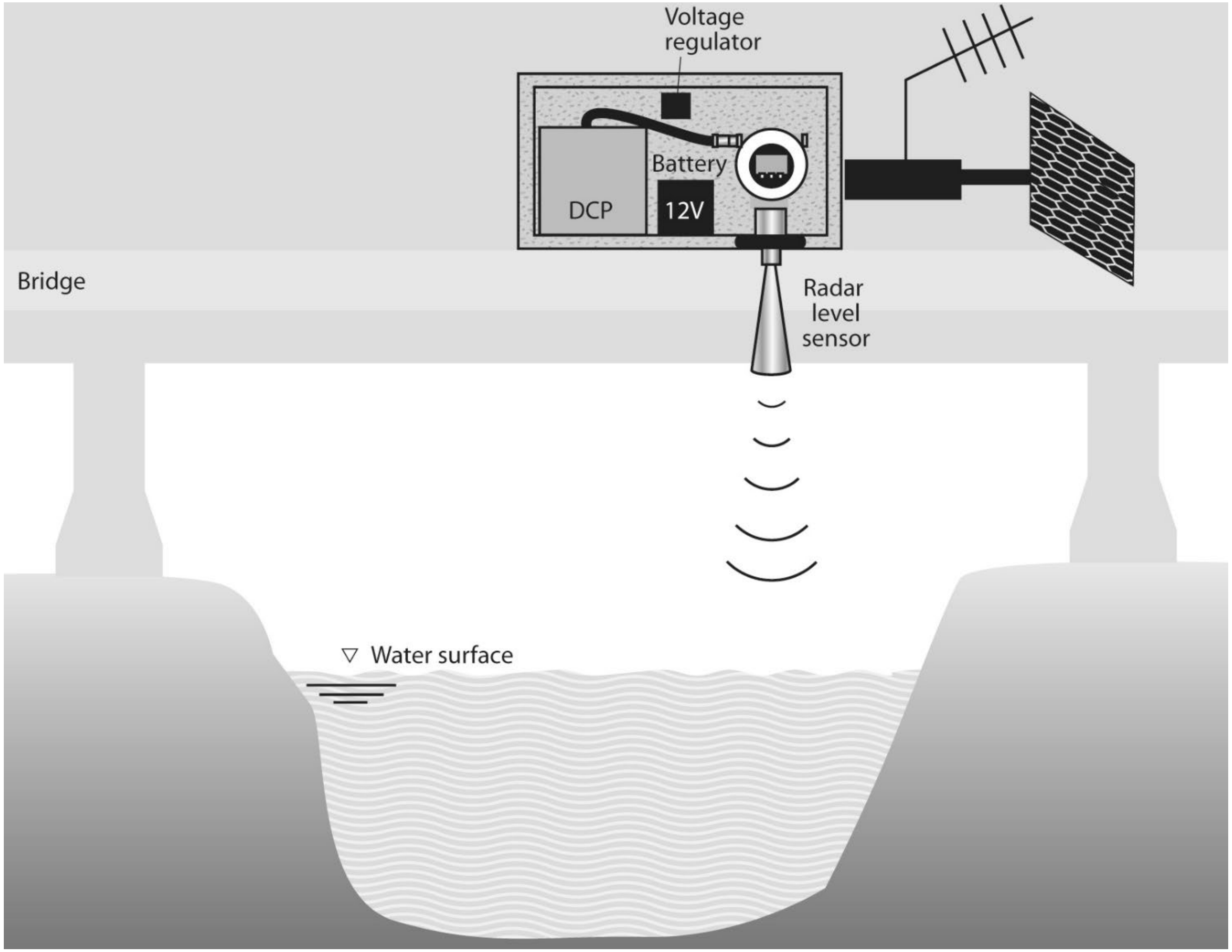
Bubble-gage Measurement

Figure 8.2 Bubble-gage installation (from Herschy, 1985a).



Radar Stage Measurement

(Figure 8.3, Gupta, 2017)



Single Stage-Discharge Relation

- Equation 8.25:
  - $Q = A\,(h \pm a)^n$ where

    Q = discharge

    H = gage height

    a = stage reading at zero flow (i.e., datum correction)

    A, n = constants to be determined for each case

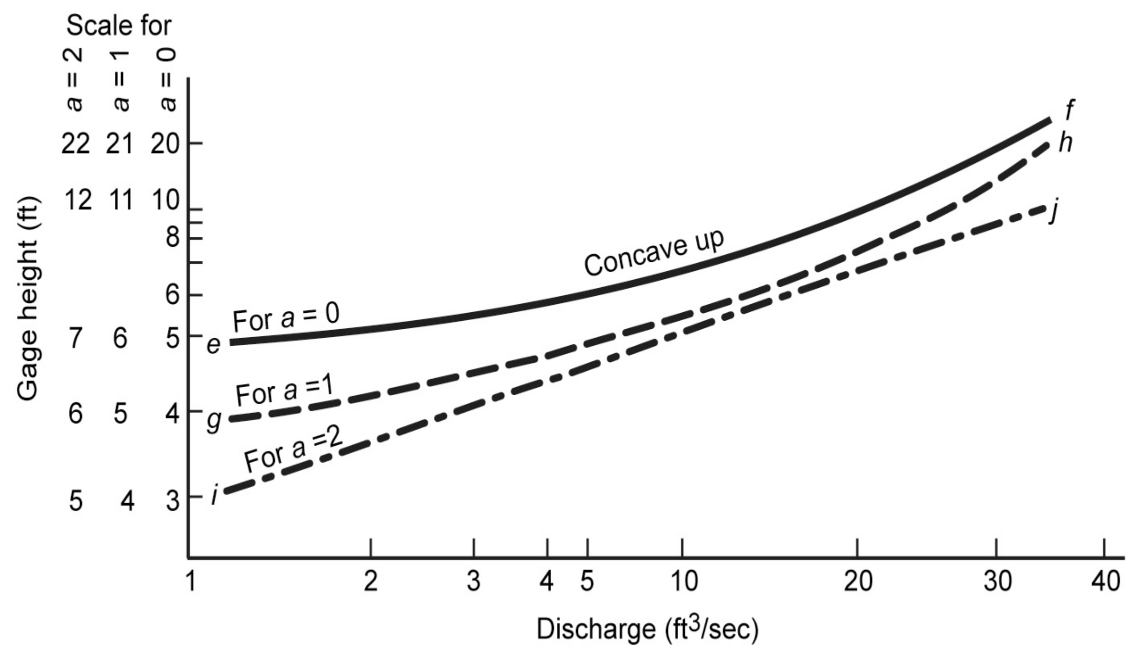
    Or

    $\text{Log } Q = n \log (h \pm a) + \log A$ (of the form $Y = M X + N$)

  - See Sections 8.19.1 and 8.21 for regression analysis

Defining “a”: Trial and Error Procedure

Figure 8.22 Trial-and-error procedure to determine the stage of zero flow.



Discharge Measurement

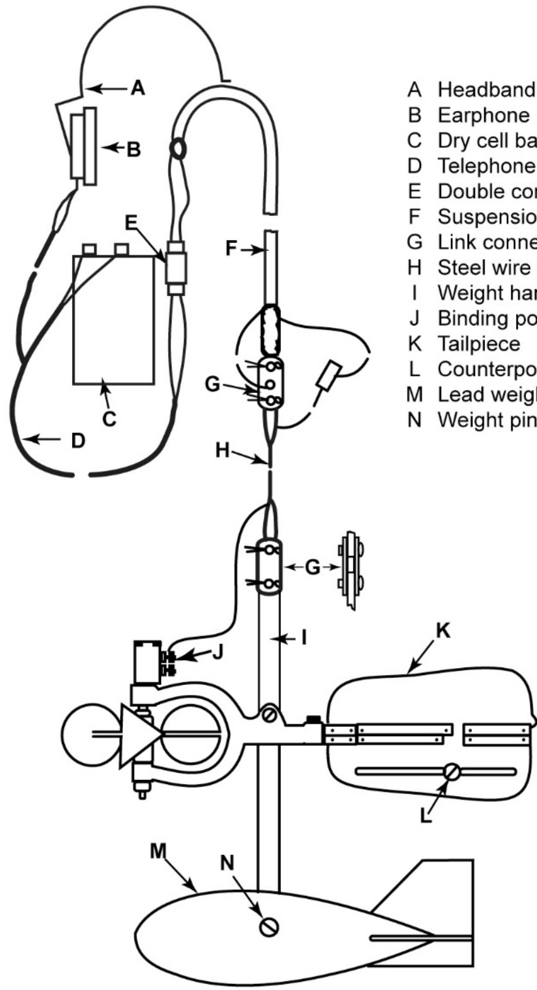
- Volumetric flow rate: $Q = \int a \cdot dv = \sum a_i \cdot v_i$ ($L\ T^{-3}$)
- Methods of measurement:
  - Current meter method (e.g., "Price"; mechanical meter, Fig. 8.4)
  - Hydroacoustic method (Acoustic Doppler velocity meter, ADCP, or sound pulse of certain frequency from and reflected back to transducer), with GPS to track the stream bottom (Fig. 8.16)
  - Ultrasonic method (ultrasonic velocity meter, UVM)
  - Electromagnetic method (based on electromagnetic induction, electrodes on banks and coils buried in stream section)

Methods to Measure Velocity and Area to Determine Discharge

- Current Meter
- Hydroacoustic (e.g., acoustic Doppler velocity meter, ADVM): sent and reflected back acoustic signal of 10 MHz frequency
- Ultrasonic (ultrasonic velocity meter, UVM)
- Electromagnetic (based on electromagnetic induction): measures electric current from water – a conductor - moving in magnetic field

Price Current Meter

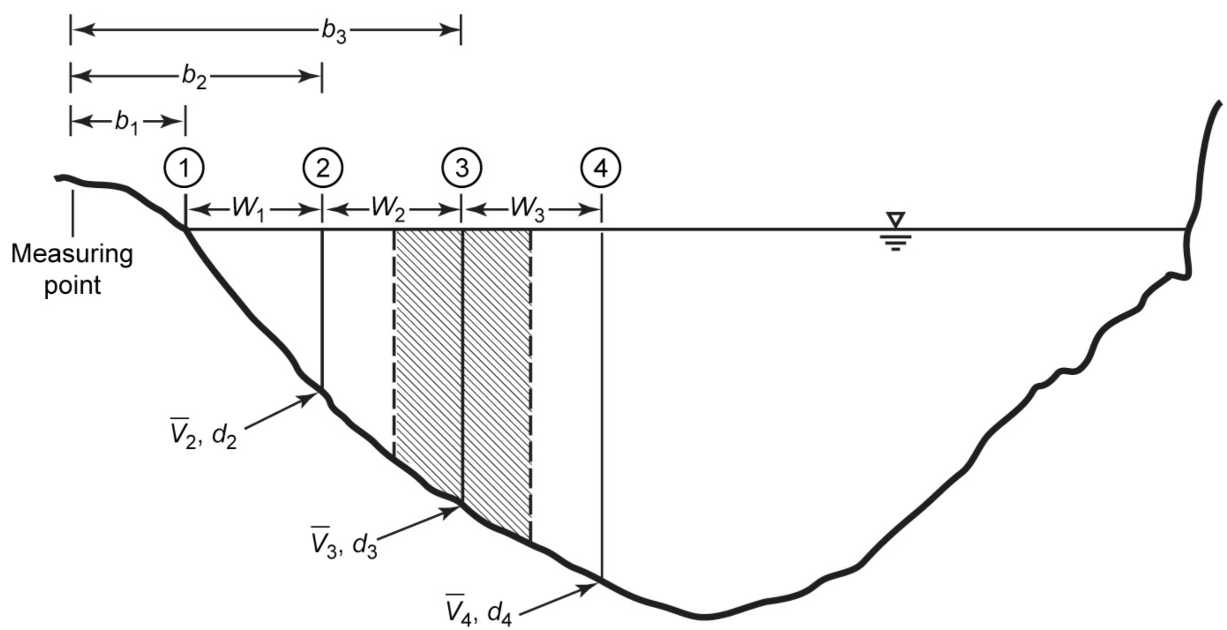
Figure 8.4 Assembly of a type AA current meter (courtesy of Geophysical Instrument and Supply Co.).



A Headband
B Earphone
C Dry cell battery (1.5 V)
D Telephone cable
E Double contact connector female
F Suspension cable, 35 ft
G Link connector
H Steel wire cable, 35 ft
I Weight hanger
J Binding post
K Tailpiece
L Counterpoise
M Lead weight
N Weight pin

Discharge Computation
by Current Meter – Mid-Section Method
Example 8.7

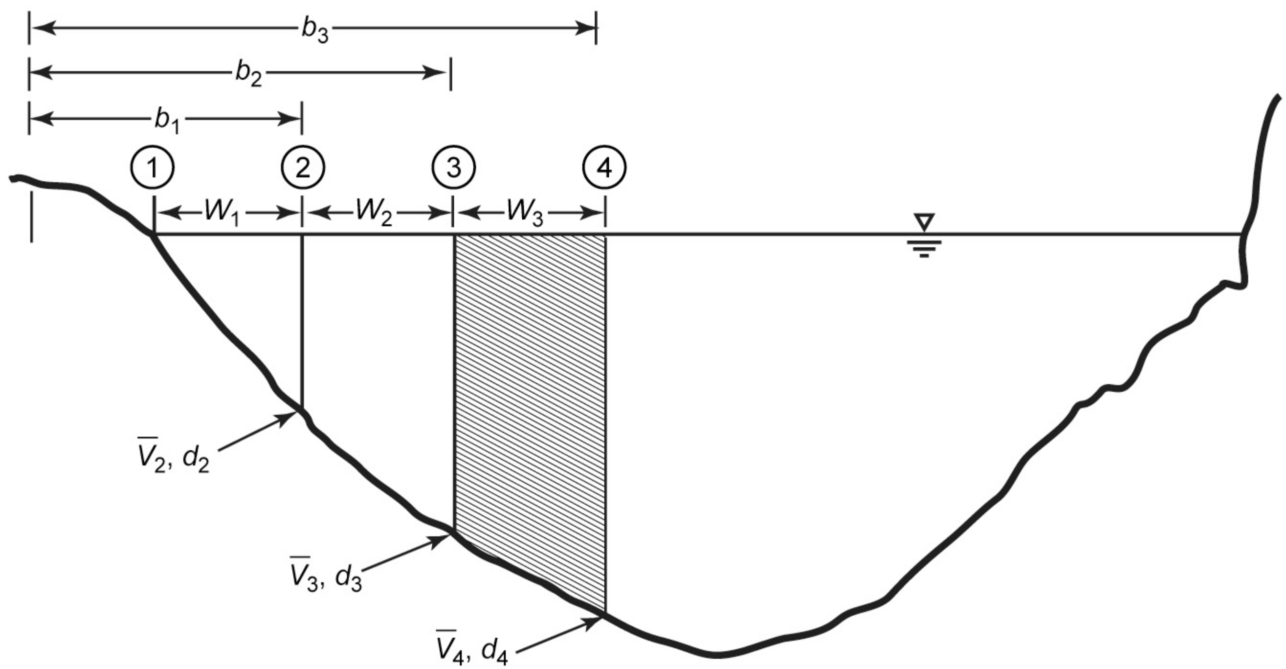
Figure 8.9 Subsection in the midsection method.



1, 2, 3, ... Stations
b_1, b_2, b_3, ... Distance from the initial point to the station (observation verticals)
d_1, d_2, d_3, ... Depth of water at the observation verticals
W_1, W_2, W_3, ... Width between successive verticals

Discharge Computation
by Current Meter – Mean-Section Method
Example 8.8

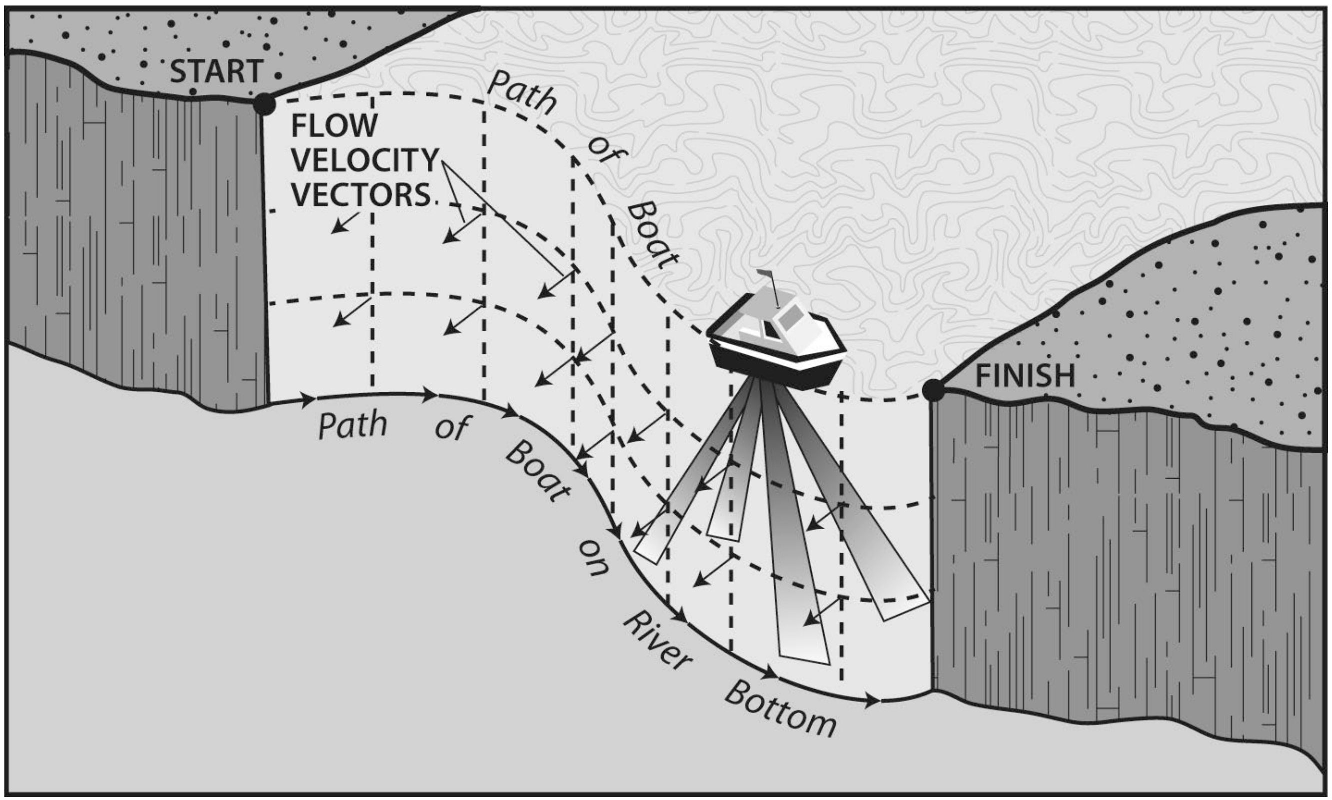
Figure 8.10 Subsection in the mean-section method.



Alternative Procedures to Cross the Stream or River: Using Current Meters

- By wading
- From a bridge
- From a cableway
- By boat
- Over ice cover

Acoustic Doppler Current Profiler, ADCP



Links to USGS: Streamflow Measurements

- **Acoustic**

https://www.usgs.gov/centers/sa-water/science/hydroacoustic-applications-technological-advancements-streamgaging-network?qt-science_center_objects=0#qt-science_center_objects

- **General**

https://www.usgs.gov/special-topic/water-science-school/science/how-streamflow-measured?qt-science_center_objects=0#qt-science_center_objects