

Example 8.7

The basic premise of discharge at the outlet being a linear response to precipitation excess (runoff volume) is illustrated by comparing three hydrographs representative of a particular 2,975-acre (1,208-ha) watershed for a specific rainfall duration. Simple triangular-shaped hydrographs are used for this hypothetical illustration. Hydrographs A, B, and C in the table and Fig. 8.9 have a time base of 5.0 hrs. There is no base flow for this watershed, since flows are zero at the beginning and end.

DISCHARGES FOR HYDROGRAPHS

Time (hrs)	Hydrograph A (ft ³ /s)	Hydrograph B (ft ³ /s)	Hydrograph C (ft ³ /s)
0	0	0	0
1	600	900	1,200
2	1,200	1,800	2,400
3	800	1,200	1,600
4	400	600	800
5	0	0	0

The runoff volume V_R is represented by the area under the hydrograph. For these hydrographs with a simple triangular shape, the runoff volume may be computed as the area of a triangle.

$$\text{Area} = 0.5 (\text{base}) (\text{height})$$

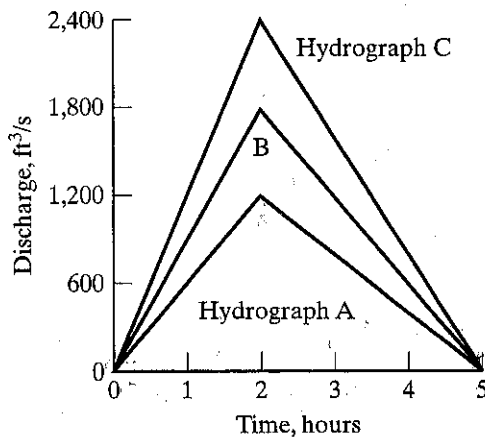


Figure 8.9 The discharges for these hydrographs from Example 8.7 vary in direct proportion to the runoff volume.

$$\text{Hydrograph A: } V_R = 0.5(1,200 \text{ ft}^3/\text{s})(5 \text{ hrs})(3,600 \text{ s/hr}) = 10,800,000 \text{ ft}^3$$

$$\text{Hydrograph B: } V_R = 0.5(1,800 \text{ ft}^3/\text{s})(5 \text{ hrs})(3,600 \text{ s/hr}) = 16,200,000 \text{ ft}^3$$

$$\text{Hydrograph C: } V_R = 0.5(2,400 \text{ ft}^3/\text{s})(5 \text{ hrs})(3,600 \text{ s/hr}) = 21,600,000 \text{ ft}^3$$

Alternatively, the hydrographs can be numerically integrated as

$$V_R = \text{area} = \sum Q_i \Delta t = \Delta t \sum Q_i$$

where Q_i are the hydrograph ordinates and Δt is the 1.0-hr time interval. For hydrograph A,

$$V_R = (600 + 1,200 + 800 + 400 \text{ ft}^3/\text{s})(3,600 \text{ s}) = 10,800,000 \text{ ft}^3$$

The volumes are converted to an equivalent depth covering the 2,975-acre watershed. For hydrograph A,

$$V_R = \left[\frac{10,800,000 \text{ ft}^3}{(2,975 \text{ ac})(43,560 \text{ ft}^2/\text{ac})} \right] (12 \text{ in./ft}) = 1.00 \text{ in.}$$

Likewise, the V_R for hydrographs B and C are computed to be 1.5 and 2.0 inches, respectively. Notice that the ordinates Q_i of the hydrographs vary in direct proportion to runoff volume. For example, hydrographs A and C have runoff volumes of 1.0 and 2.0, respectively. Thus, for any time t , the Q_i for hydrograph C is twice the value for hydrograph A.

SOURCE: Wurbs & James, 2002