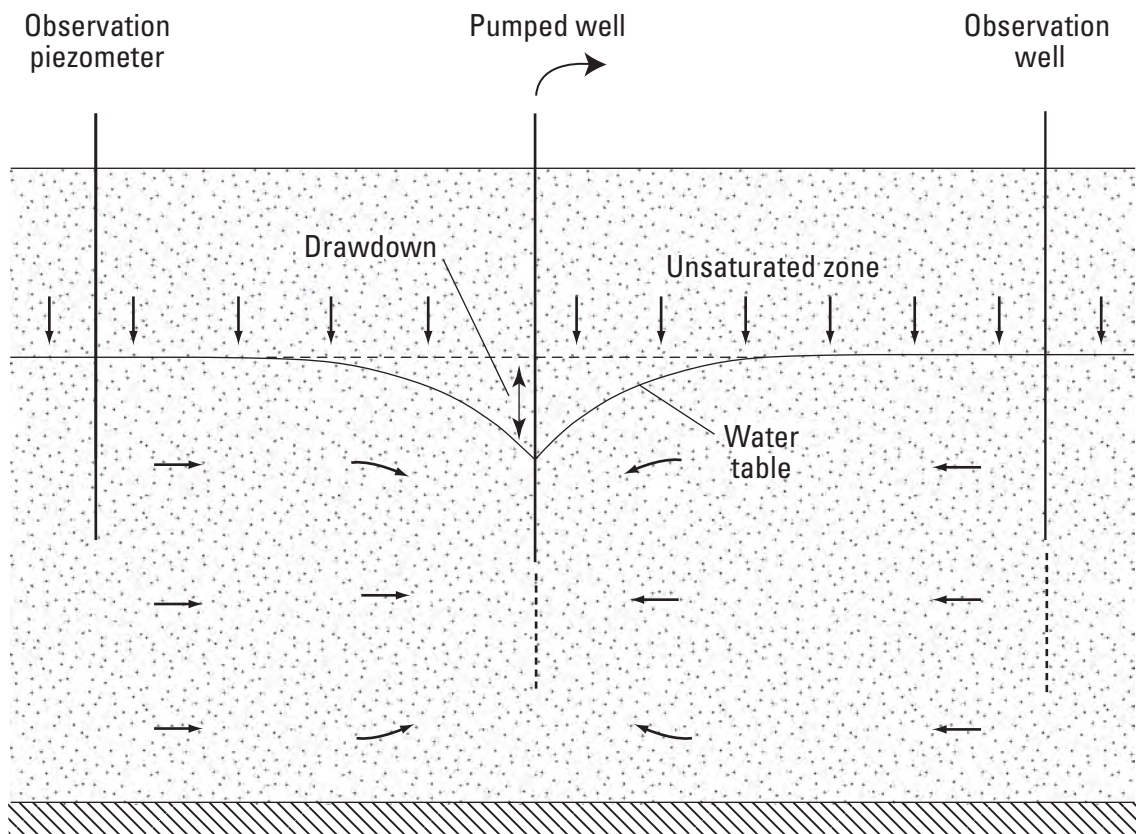


Groundwater Resources Program

WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers with Alternative Representations of Drainage from the Unsaturated Zone



Techniques and Methods 3–B9

Cover. Schematic diagram of two-dimensional, axial-symmetric flow in a water-table aquifer with one-dimensional, vertical flow in the unsaturated zone.

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By Paul M. Barlow and Allen F. Moench

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Techniques and Methods 3–B9

**U.S. Department of the Interior
U.S. Geological Survey**

U.S. Department of the Interior
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U.S. Geological Survey
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Preface

This report describes an updated version of the WTAQ computer program first published in 1999. The program simulates axial-symmetric flow to a well pumping from a confined or unconfined (water-table) aquifer. The performance of the program has been tested in a variety of applications, some of which are documented in this report. Future applications, however, might reveal errors that were not detected in the test simulations. Users are requested to notify the U.S. Geological Survey of any errors found in this report or the computer program by using the address on the inside of the back cover of the report. Updates might occasionally be made to both the report and to the computer program. Users can check for updates on the Internet at <http://water.usgs.gov/software/lists/groundwater/>.

Contents

Abstract.....	1
Introduction.....	1
Representation of Drainage From the Unsaturated Zone.....	2
Overview of the Analytical Model of Mathias and Butler.....	2
Implementation of the Analytical Solution in WTAQ.....	5
Evaluation of Revised Program.....	6
Sample Problem.....	9
Acknowledgments.....	12
References Cited.....	12
Appendix 1. Data-Input Instructions for WTAQ Version 2.....	15

Figures

1. Diagram showing two-dimensional, axial-symmetric flow in a water-table aquifer with one-dimensional, vertical flow in the unsaturated zone.....	3
2. Diagram showing finite-diameter, partially penetrating pumped well in a water-table aquifer.....	5
3. Graph showing dimensionless drawdown as a function of dimensionless time for several values of the dimensionless soil-moisture retention exponent.....	7
4. Graph showing dimensionless drawdown as a function of dimensionless time for several values of the dimensionless relative hydraulic-conductivity exponent.....	8
5. Graph showing dimensionless drawdown as a function of dimensionless time for several values of dimensionless thickness of the unsaturated zone.....	8

6.	Diagram showing vertical section of the aquifer at the field site showing the positions of the screened interval of the pumped well, observation piezometers, and neutron-access tube MBN-5	9
7.	(A) Schematic diagram of the soil-moisture content profile at the field site prior to pumping and (B) graph showing background volumetric soil-moisture content above initial water table for neutron-access tube MBN-5 compared with an approximate fit to equation 1 with soil-moisture retention exponent equal to 5 m^{-1}	10
8.	Graphs showing measured and simulated drawdown at several observation piezometers at the field site: (A) piezometers P14 and WD1A; (B) WD2A and WD4A; and (C) P17, P4, and P5	11
9.	Data-input file for sample problem	28
10.	Output-results file for sample problem	33

Table

1.	Location, depth, and radius of each observation piezometer for sample problem	9
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Conversion Factors

Multiply	By	To obtain
meter (m)	3.281	foot (ft)
cubic meter per minute (m^3/min)	35.31	cubic foot per minute (ft^3/min)
liter per minute (L/min)	0.2642	gallon per minute (gal/min)
meter per minute (m/min)	3.281	foot per minute (ft/min)

WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers with Alternative Representations of Drainage from the Unsaturated Zone

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Abstract

The computer program WTAQ simulates axial-symmetric flow to a well pumping from a confined or unconfined (water-table) aquifer. WTAQ calculates dimensionless or dimensional drawdowns that can be used with measured drawdown data from aquifer tests to estimate aquifer hydraulic properties. Version 2 of the program, which is described in this report, provides an alternative analytical representation of drainage to water-table aquifers from the unsaturated zone than that which was available in the initial versions of the code. The revised drainage model explicitly accounts for hydraulic characteristics of the unsaturated zone, specifically, the moisture retention and relative hydraulic conductivity of the soil. The revised program also retains the original conceptualizations of drainage from the unsaturated zone that were available with version 1 of the program to provide alternative approaches to simulate the drainage process. Version 2 of the program includes all other simulation capabilities of the first versions, including partial penetration of the pumped well and of observation wells and piezometers, well-bore storage and skin effects at the pumped well, and delayed drawdown response of observation wells and piezometers.

Introduction

Aquifer-test analyses are one of the most widely used methods for estimating hydraulic properties such as transmissivity, vertical and horizontal hydraulic conductivity, storativity, and specific yield. Several computer programs are available to assist with aquifer-test analysis for a wide range of aquifer conditions and well-design factors. WTAQ is a publicly available program developed by the U.S. Geological Survey for analysis of aquifer tests completed in confined and unconfined (water-table) aquifers (Barlow and Moench, 1999). WTAQ is based on an analytical model developed by Moench (1997; later extended by Moench and others, 2001) for axial-symmetric flow to a partially penetrating, finite-diameter well that pumps water from a homogeneous, anisotropic aquifer. The model accounts for well-bore storage and skin at the pumped well and delayed drawdown response at an observation well or piezometer. WTAQ calculates dimensionless or dimensional drawdowns that can be used with measured drawdowns at observation points to estimate aquifer hydraulic properties. Since its initial release in 1999, WTAQ has been used for the determination of aquifer properties (Kollet and Zlotnik, 2005; Barrash and others, 2006; Endres and others, 2007) and for benchmark testing of numerical models of groundwater flow (Clemons, 2005; Langevin, 2008). As part of a recent textbook on aquifer-test modeling, Walton (2007) provides examples of the use of WTAQ for analysis of aquifer tests in confined and unconfined aquifers.

The analytical model developed by Moench combines and extends the work of Boulton (1954, 1963) and of Neuman (1972, 1974) to account for the release of water from the unsaturated zone above the water table. In Boulton's approach, drainage from the unsaturated zone is assumed to occur gradually in a manner that varies exponentially with time in response to a unit decline in the elevation of the water table. In Neuman's approach, water is assumed to be released instantaneously (and completely) from the zone above the water table in response to a lowering of the water table. Whereas Boulton's approach is based on a single parameter in the exponential drainage function, Moench's approach allows the user to specify multiple parameters to model the drainage process. Moench has referred to these constants as empirical fitting parameters. Properly evaluated, two or three empirical fitting parameters result in improved matches between simulated and measured drawdowns.

2 WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers

Several studies over the past 25 years have demonstrated the importance of flow processes in the unsaturated zone to the interpretation of aquifer tests completed in water-table aquifers. These include field investigations reported by Nwankwor and others (1984, 1992) and Bevan and others (2005) and analytical- and numerical-modeling investigations described by Akindunni and Gillham (1992), Narasimhan and Zhu (1993), Moench (1995, 2003, 2004), Halford (1997), Moench and others (2001), and Endres and others (2007). The results of these studies point to limitations in the way the Boulton and Neuman models account for drainage from the unsaturated zone.

In an important breakthrough, Mathias and Butler (2006) developed a solution for flow to a well in a water-table aquifer that incorporates explicit representations of unsaturated-zone hydraulic characteristics, specifically, the moisture retention and relative hydraulic conductivity of the soil. The solution was designed to be incorporated into existing analytical solutions such as those developed by Moench. As such, it was used by Moench (2008) in an analysis of a 7-day aquifer test conducted at the Borden research site in Ontario, Canada, wherein measured data included not only drawdown in piezometers but also volumetric soil moisture at various times and distances from the pumped well (Bevan and others, 2005). Moench (2008) demonstrated that the Mathias and Butler model yielded a set of aquifer hydraulic parameters that was consistent with that determined by use of his modified Boulton model with multiple fitting parameters (Moench, 2004) and with results obtained with a numerical model of variably saturated flow (VS2DT; Lappala and others, 1987, Healy, 1990). Separately, Mathias and Butler (2006, figs. 7 and 8) demonstrated by analysis of two unconfined aquifer tests that their model was better able to simulate drawdowns at the Borden site than the Moench (1995) model and was able to simulate drawdown at the Cape Cod site about as well as the model of Moench (2004), particularly during the intermediate time period when drainage from the unsaturated zone is important.

Mishra and Neuman (2010) have developed a solution that improves upon a model by Tartakovsky and Neuman (2007) by characterizing relative hydraulic conductivity and water content by two separate exponents and allowing for finite thickness of the unsaturated zone, as accomplished by Mathias and Butler (2006). Unlike the Mathias and Butler (2006) solution, the Mishra and Neuman model accounts for horizontal flow in the unsaturated zone. However, it has the disadvantage that it does not account for storage and skin at the pumped well or delayed piezometer response. Because of this, the model does not properly simulate early-time data and cannot be used to correctly estimate specific storage.

This report documents an update to the WTAQ computer program to include the solution of Mathias and Butler (2006). The updated code is called WTAQ version 2. The report includes a description of the analytical model of Mathias and Butler and its implementation in WTAQ, updated instructions for preparing input files for WTAQ, and a sample problem that demonstrates use of the code. Users of the updated program must refer to the initial WTAQ documentation (Barlow and Moench, 1999) for detailed information on the input parameters and use of the program. All of the capabilities that were available in version 1 of the program are also available in version 2, although the format of the data-input files for the program has changed. Unless otherwise noted, all subsequent references to WTAQ are to version 2 of the program.

Representation of Drainage From the Unsaturated Zone

This section provides an overview of the analytical model of Mathias and Butler (2006), steps taken to integrate the model into WTAQ, and an evaluation of the solution for hypothetical aquifer conditions to demonstrate behavior of the solution. A full description of the derivation and solution of the model by Mathias and Butler, which is beyond the scope of this report, is provided in their paper. A schematic figure illustrating the underlying conditions for the analytical model is provided in figure 1.

Overview of the Analytical Model of Mathias and Butler

The analytical model of Mathias and Butler (2006) extends the work of Boulton (1954, 1963), Dagan (1967), Neuman (1972, 1974), Kroszynski and Dagan (1975), Moench (1995, 1997), and Moench and others (2001) for axial-symmetric flow to a well that pumps from a water-table aquifer. As with the earlier solutions, that of Mathias and Butler is based on a number of simplifying assumptions for the aquifer:

1. The aquifer is homogeneous, of infinite lateral extent, horizontal, and of uniform thickness.
2. The aquifer can be anisotropic provided that the principal directions of the hydraulic-conductivity tensor are parallel to the radial (r) and vertical (z) coordinate axes.
3. Vertical flow across the lower boundary of the aquifer is negligible (that is, the lower boundary is impermeable).
4. A well discharges at a constant rate from a specified zone below an initially horizontal water table.

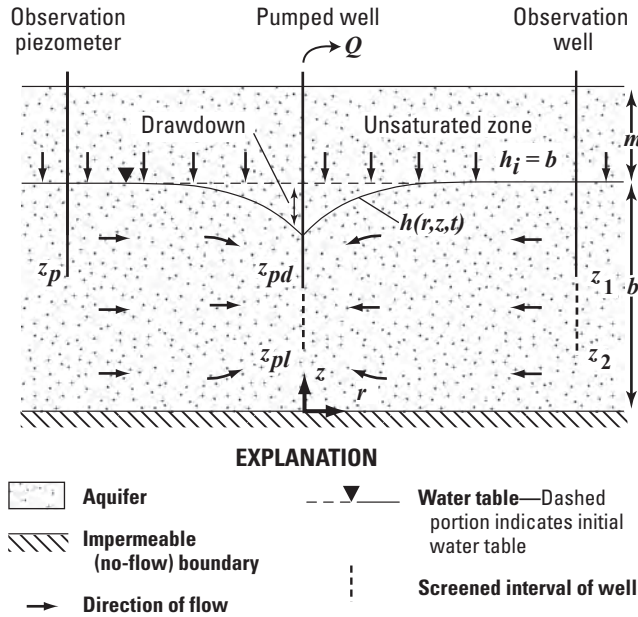


Figure 1. Two-dimensional, axial-symmetric flow in a water-table aquifer with one-dimensional, vertical flow in the unsaturated zone. (b , initial saturated thickness of aquifer; m , initial thickness of unsaturated zone; $h(r, z, t)$, head in aquifer at radial distance r from axis of pumped well and height z above the base of the aquifer at time t ; h_i , initial head in aquifer; Q , pumping rate of well; z_{pd} , z_{pb} , depth below initial water table to the top and bottom, respectively, of the screened interval of the pumped well; z_1 , z_2 , depth below initial water table to the top and bottom, respectively, of the screened interval of the observation well; z_p , depth below initial water table to center of piezometer)

5. The porous medium and fluid are slightly compressible and have constant physical properties.
6. The pumped well and observation wells or piezometers are infinitesimal in diameter.
7. The pumped well fully penetrates the aquifer.
8. The change in saturated thickness of the aquifer due to pumping is small compared with the initial saturated thickness.

The several analytical models differ in their treatment of drainage to the pumped aquifer from the overlying unsaturated zone. As described previously, the models of Boulton assume a gradual release of water from the unsaturated zone, whereas those of Neuman assume an instantaneous and complete release of water in response to a lowering of the water table. Although these models account for the effects of gravity drainage from the unsaturated zone on the response of water levels in the saturated zone, none of them, with the exception of Kroszynski and Dagan (1975), explicitly represent flow within the unsaturated zone or include unsaturated-zone hydraulic characteristics.

The Mathias and Butler model combines the two-dimensional, partial-differential equation of flow within an unconfined aquifer with the one-dimensional form of Richards (1931) equation for vertical flow in a homogeneous unsaturated zone. An important difference between modeling flow in the unsaturated zone and modeling flow in the saturated zone is that it is necessary to specify how soil-moisture content and relative hydraulic conductivity of the unsaturated zone vary as a function of pressure head. Expanding upon the work of Kroszynski and Dagan (1975), Mathias and Butler assumed that both the soil-moisture content and relative hydraulic conductivity can be described by exponential functions of pressure head. Following the approach of Gardner (1958), these functions are written

$$S_e(\psi) = e^{a_c(\psi - \psi_s)} \quad (1)$$

and

$$k_{rel}(\psi) = e^{a_k(\psi - \psi_s)}, \quad (2)$$

where

- $S_e(\psi)$ and $k_{rel}(\psi)$ are the effective-saturation and relative hydraulic-conductivity functions (dimensionless), respectively;
- ψ is pressure head in the unsaturated zone (units of length; $\psi < 0$);
- ψ_s is pressure head at which the aquifer starts to desaturate (that is, the air-entry pressure head) (units of length; $\psi_s < 0$);
- a_c is the soil-moisture retention exponent (units of inverse length); and
- a_k is the relative hydraulic-conductivity exponent (units of inverse length).

4 WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers

Relative hydraulic conductivity ($a_k k_{rel}(\psi)$) is defined as the ratio of unsaturated hydraulic conductivity (which is a function of pressure head) to saturated hydraulic conductivity. Effective saturation ($S_e(\psi)$) is defined as

$$\frac{\theta - \theta_r}{\varphi - \theta_r},$$

where

- θ is the volumetric soil-moisture content,
- θ_r is the residual soil-moisture content, and
- φ is the soil porosity.

The sample problem described later in this report illustrates how a_c and ψ_s can be estimated from measurements of volumetric soil-moisture content.

The relative hydraulic-conductivity and effective-saturation functions are related by (Mathias and Butler, 2006, eq. 6)

$$k_{rel}(S_e) = S_e^{a_k/a_c}. \quad (3)$$

By analogy to the models of relative permeability developed by Burdine (1953) and Mualem (1976), the exponent a_k / a_c should be greater than or equal to 1.0 and as a result, $a_k \geq a_c$, which is a necessary condition for the analytical solution implemented in WTAQ (S.A. Mathias, Durham University, United Kingdom, written commun., 2010).

Mathias and Butler (2006) derive their solution to the coupled-flow model by linearizing the Richards equation. This approach requires an extension of assumption 8 listed above; specifically, that the changes in the initial thickness of the unsaturated zone (m) and the initial saturated thickness of the aquifer (b) due to pumping are small compared to the initial conditions. The solution to the coupled-flow model results in an analytical expression for drainage from the unsaturated zone (equation 42 in Mathias and Butler) that can be incorporated into the Laplace-transform solutions for flow to a well in an unconfined aquifer that are the basis of version 1 of WTAQ (specifically, equation 24 in Moench, 1997; equation 20 in Moench and others, 2001).

The approach taken by Mathias and Butler (2006) is an extension of earlier work by Kroszynski and Dagan (1975) and is similar to the approach taken by Tartakovsky and Neuman (2007). However, unlike the models of Kroszynski and Dagan (1975) and Tartakovsky and Neuman (2007), the model of Mathias and Butler does not require that the soil-moisture retention and relative hydraulic-conductivity exponents be equal. Moench (2008) has shown that the assumption $a_c = a_k$ is unrealistic for field applications. The Mathias and Butler model also accounts for elastic storage in the saturated zone and allows for a finite thickness of the unsaturated zone, neither of which was accounted for by Kroszynski and Dagan (1975). Unlike the model of Tartakovsky and Neuman, however, the Mathias and Butler model assumes that horizontal flow in the unsaturated zone can be neglected.

Implementation of the Analytical Solution in WTAQ

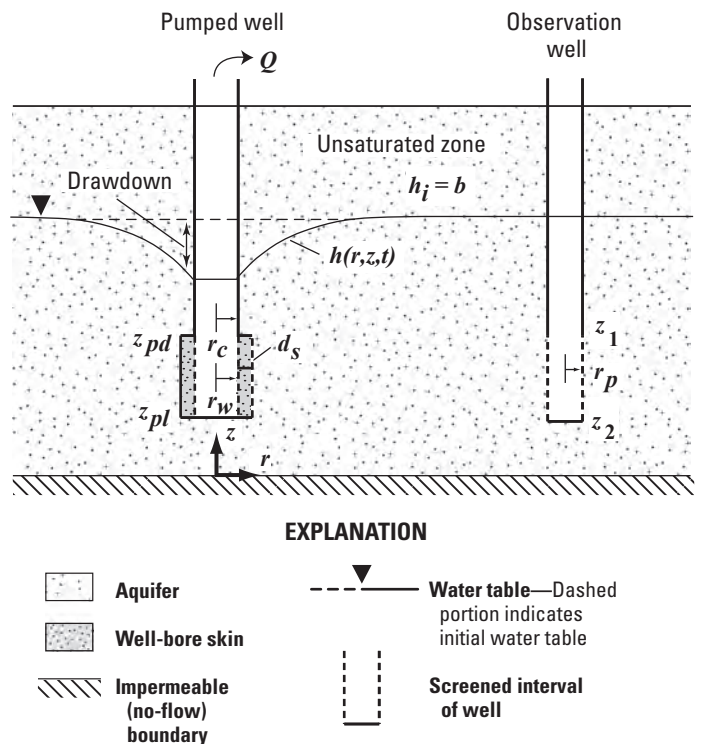
Because the drainage function derived by Mathias and Butler can be incorporated into the solutions derived by Moench, all of the simulation capabilities that are provided by the first version of WTAQ are included with version 2 of the program. These include partial penetration of the pumped well and of observation wells and piezometers, well-bore storage and skin effects at the pumped well, and delayed drawdown response of observation wells and piezometers (these conditions are shown schematically in figure 2). Barlow and Moench (1999) provide a full description of all these simulation capabilities and a summary of the additional simplifying assumptions necessary for the simulation of these conditions.

WTAQ uses numerical-inversion techniques to invert the Laplace-transform (LT) analytical solutions developed by Moench (2008) and Mathias and Butler (2006) into the real-time domain. All LT solutions provided in version 1 of the program were inverted by use of the Stehfest (1970) algorithm. In version 2, however, either the Stehfest (1970) or de Hoog (de Hoog and others, 1982) algorithms can be employed at the discretion of the user. A new variable (ISOLN) has been added to WTAQ to specify which inversion algorithm is to be used. For simulation of confined aquifers using WTAQ versions 1 or 2, the Stehfest algorithm must be used. For simulation of water-table aquifers with either the Boulton or Neuman models for drainage from the unsaturated zone, the Stehfest algorithm is recommended, although either the Stehfest or de Hoog algorithms can be used if a comparison of results is desired. For simulation of drainage that is based on the Mathias and Butler model, the de Hoog algorithm must be used because the LT solution of Mathias and Butler is expressed in complex notation. (Use of the Stehfest algorithm requires the LT solution be expressed in real notation.) Both the Stehfest and de Hoog algorithms have occasional stability problems that are likely to occur at early time (that is, large values of the LT variable). If they occur, they may be a consequence of the choice of algorithm options (input variables that must be specified for use of each algorithm are described in the appendix).

WTAQ version 2 consists of four FORTRAN-language computer programs that are linked during program compilation:

1. The main program `wtaq.v2.f`, which organizes the tasks completed by WTAQ and calls many of the subroutines;
2. Program `subs.io.f`, which comprises subroutines related to opening the input, output results, and plot files, reading of input data, and writing of a program banner to the results file;
3. Program `subs.inverse.f`, which comprises subroutines related to the Stehfest and de Hoog numerical-inversion methods for solution of the analytical equations; and
4. Program `subs.besselmb.f`, which comprises the Bessel functions used in the equations of Mathias and Butler (2006) that were developed by Amos (1986).

Figure 2. Finite-diameter, partially penetrating pumped well in a water-table aquifer. (b , initial saturated thickness of aquifer; d_s , thickness of the well-bore skin; $h(r,z,t)$, head in aquifer at radial distance r from axis of pumped well and height z above the base of the aquifer at time t ; h_i , initial head in aquifer; Q , pumping rate of well; r_c , r_p , inside radius of the pumped well and observation well, respectively, in the interval where water levels are changing during pumping; r_w , radius of the screened interval of the pumped well; z_{pd} , z_{pl} , depth below initial water table to the top and bottom, respectively, of the screened interval of the pumped well; z_1 , z_2 , depth below initial water table to the top and bottom, respectively, of the screened interval of the observation well)



Evaluation of Revised Program

Several tests were made to confirm that the revised program works correctly. First, three of the sample problems that were described in Barlow and Moench (1999) and are distributed with the program (problems SP1, SP2, and TEST1) were rerun with the revised program. Each of these sample problems simulates water-table aquifers with instantaneous drainage above the declining water table; in each case, the Stehfest algorithm was used to numerically invert the Laplace-transform analytical solutions. Drawdowns calculated with the revised program were identical to those calculated with version 1 of the program for several simulated conditions that included simulation of gradual drainage from the unsaturated zone (that is, variable IDRA set to 1). The input file for each sample problem was then modified such that the de Hoog algorithm was used to calculate drawdowns. Results from these simulations were nearly identical to those calculated with the Stehfest algorithm, indicating that either algorithm adequately simulates drawdown in the water-table aquifers.

Three sets of tests were then carried out to verify that the analytical solution of Mathias and Butler (2006) was correctly implemented in the code. The tests were based on simulated conditions that closely follow the hypothetical aquifer and unsaturated-zone conditions described in Mathias and Butler. The simulated water-table aquifer has an initial saturated thickness of 10 m. The vertical (K_z) and radial (K_r) hydraulic conductivity of the aquifer are assumed to be equal, so that $K_z / K_r = 1$. Values of specific storage (S_s) and specific yield (S_y) of the aquifer were chosen such that the dimensionless variable $\sigma (S_s b / S_y)$ has a value of 1.0×10^{-3} . The pumped well is assumed to fully penetrate the aquifer. Drawdowns were calculated for an observation piezometer that is located 10 m from the pumped well and is open to the aquifer at the water table.

Results of the test simulations are shown as dimensionless drawdowns calculated for several values of dimensionless time. Dimensionless drawdown and dimensionless time are defined for WTAQ in Barlow and Moench (1999, p. 25). Dimensionless time-drawdown curves also were calculated for assumed Theis-type confined-aquifer conditions for comparison to the water-table aquifer solutions. Two confined-aquifer conditions were simulated, the first for dimensionless time as a function of storativity of the aquifer (S) and the second for dimensionless time as a function of specific yield (S_y). The resulting time-drawdown curves are referred to as “Theis (S)” and “Theis (S_y)” on figures 3–5.

Three sets of tests were run to evaluate the effects of the three parameters that are required for the model of Mathias and Butler (2006): the soil-moisture retention exponent, a_c ; the relative hydraulic-conductivity exponent, a_k ; and the initial thickness of the unsaturated zone at the start of the aquifer test, m . In the first two sets of tests, the unsaturated zone is assumed to have an infinite thickness. This assumption is made to simplify the simulation process, which is possible because the solution of Mathias and Butler is easier to evaluate if the unsaturated zone has an infinite thickness (see Mathias and Butler, 2006, equation 46 and preceding discussion). In all cases tested, m was specified to be greater than or equal to a_c .

The first set of tests was done to evaluate the sensitivity of calculated drawdown to the soil-moisture retention exponent a_c . Values of a_c were selected such that values of the dimensionless soil-moisture retention exponent a_{Dc} ($a_{Dc} = a_c b$) were equal to 0.1, 1.0, 10.0, and 1,000. In each simulation, the value of the relative hydraulic-conductivity exponent a_k was set equal to a_c . The results of the test simulations are shown in figure 3 and closely follow those shown in figure 2 of Mathias and Butler (2006) for similar simulated conditions. The results for $a_{Dc} = 1,000$ are shown on the figure as $a_{Dc} = \infty$ and indicate that as the value of the soil-moisture retention exponent becomes very large, drainage from the unsaturated zone becomes instantaneous. This conclusion was confirmed by simulating the same aquifer conditions with an assumption of instantaneous drainage from the unsaturated zone (that is, the Neuman approach); results for the simulation overlie those shown on the figure for $a_{Dc} = \infty$. As noted by Mathias and Butler, as a_{Dc} decreases, the unsaturated zone becomes harder to drain and drawdown in the aquifer is larger at early times than is that calculated for larger values of a_{Dc} . As dimensionless time increases, all the drawdown curves approach the “late-time” Theis solution (that is, the Theis (S_y) curve), which indicates that as time progresses, the rate of drainage from the unsaturated zone approaches the rate of decline in the water table.

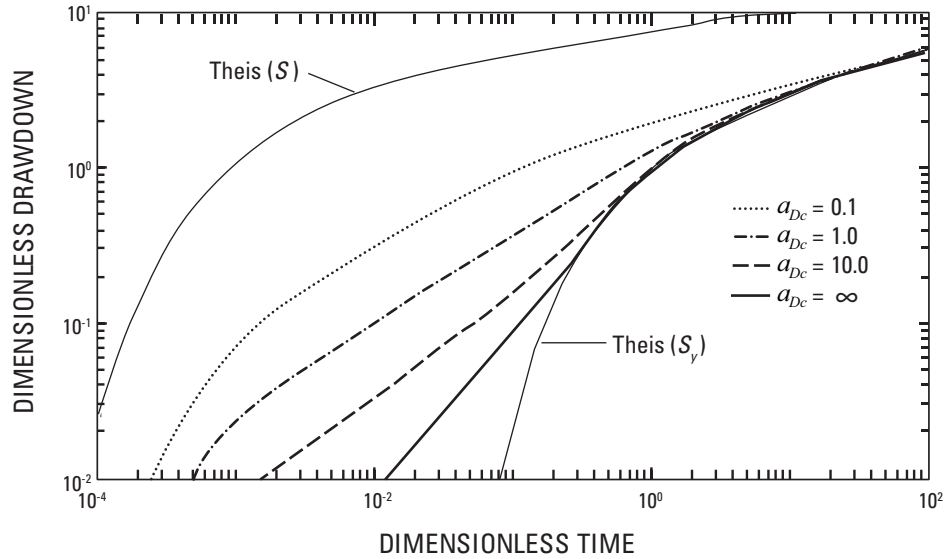


Figure 3. Dimensionless drawdown as a function of dimensionless time for several values of the dimensionless soil-moisture retention exponent (a_{Dc}). For the simulations shown, the values of the soil-moisture retention exponent and relative hydraulic-conductivity exponent were set equal to one another, and the unsaturated zone was assumed to have an infinite thickness. (The two curves labeled Theis (S) and Theis (S_y) are results for confined-aquifer conditions with dimensionless time as a function of storativity and specific yield, respectively.)

The second set of tests evaluated the sensitivity of calculated drawdowns to the relative hydraulic-conductivity exponent a_k . As with the first set of simulations, the thickness of the unsaturated zone was assumed to be infinite. Values of a_k were selected such that the values of the dimensionless relative hydraulic-conductivity exponent a_{Dk} ($a_{Dk} = a_k b$) were equal to 1.0, 100, 1,000, and 10,000. In each simulation, the value of the soil-moisture retention exponent a_c was set equal to 0.1 m^{-1} ($a_{Dc} = 1.0$). The results of the test simulations are shown in figure 4 and closely follow those shown in figure 4 of Mathias and Butler for similar simulated conditions. (Note: Two of the curves on figure 4 of the Mathias and Butler paper appear to be mislabeled. The curve shown as $a_{Dk} = 10^1$ is apparently 10^2 and that labeled 10^5 is apparently 10^4). As the value of a_k is increased, there is an exponential decline in the value of the relative hydraulic conductivity (eq. 2), and water drains more slowly from the unsaturated zone; for extremely large values of a_k , the aquifer behaves like a confined aquifer in which there is no drainage from the unsaturated zone (that is, the curves approach the “Theis (S)” curve).

In the final set of simulations, the unsaturated zone was assumed to have a finite thickness. Values of the initial thickness of the unsaturated zone (m) were varied such that the dimensionless thickness of the unsaturated zone ($m_D = m/b$) had simulated values of 0.001, 0.01, 0.1, and 100 (the value of 100 was assumed to equal infinity, ∞). In each simulation, the values of a_{Dc} and a_{Dk} were held constant at 1.0. The results of the simulations are shown in figure 5 and are equivalent to those provided in Mathias and Butler (2006, fig. 3) for the same simulation conditions. The curve for an infinitely thick unsaturated zone ($m_D = \infty$) is equivalent to that for $a_{Dk} = 1.0$ in figure 4. As the initial thickness of the unsaturated zone decreases, less water is available for drainage from the unsaturated zone, and drawdowns approach those for a confined aquifer with no drainage of aquifer porosity (that is, drawdowns approach the “Theis (S)” curve).

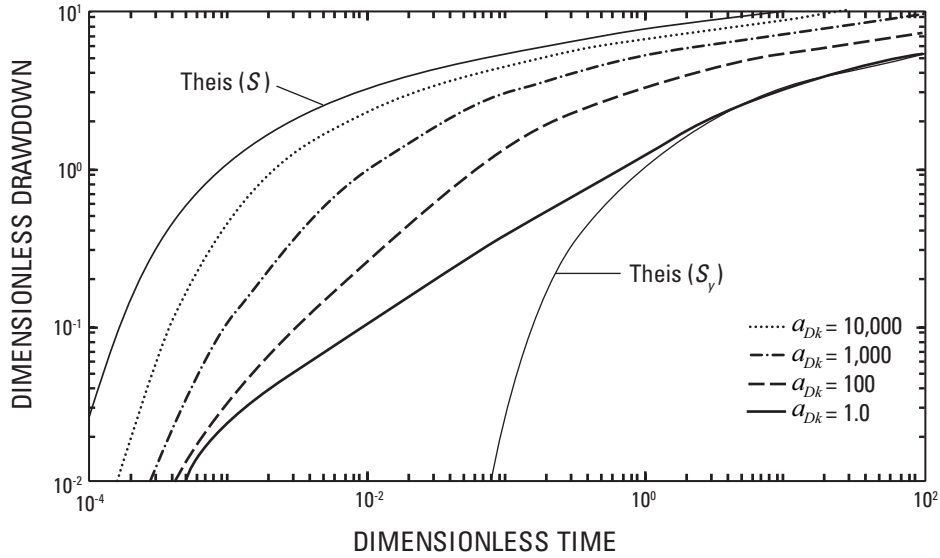


Figure 4. Dimensionless drawdown as a function of dimensionless time for several values of the dimensionless relative hydraulic-conductivity exponent (a_{Dk}). For the simulations shown, the value of the dimensionless soil-moisture retention exponent was set equal to 1.0, and the unsaturated zone was assumed to have an infinite thickness. (The two curves labeled Theis (S) and Theis (S_y) are results for confined-aquifer conditions with dimensionless time as a function of storativity and specific yield, respectively.)

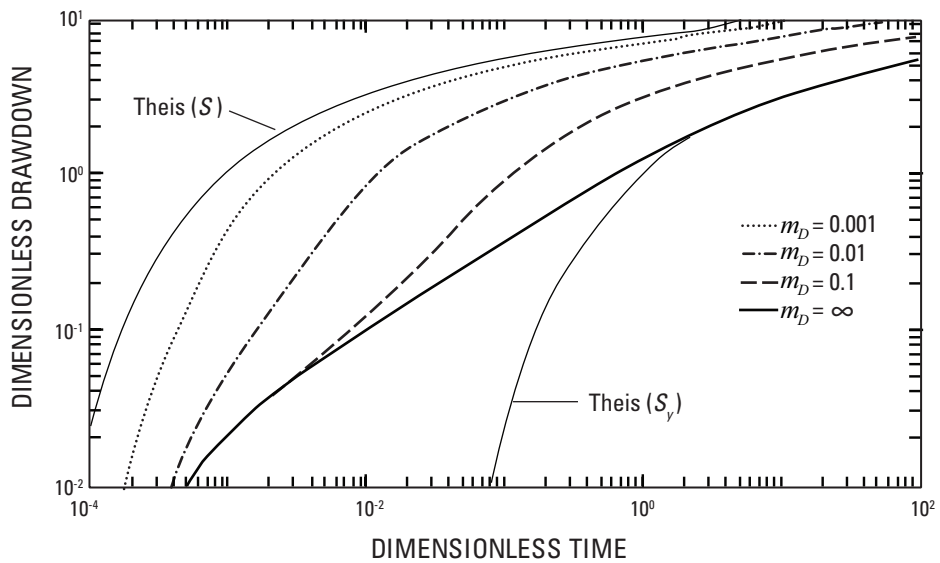


Figure 5. Dimensionless drawdown as a function of dimensionless time for several values of dimensionless thickness of the unsaturated zone (m_D). For the simulations shown, the values of the soil-moisture retention exponent and relative hydraulic-conductivity exponent were set equal to 1.0. (The two curves labeled Theis (S) and Theis (S_y) are results for confined-aquifer conditions with dimensionless time as a function of storativity and specific yield, respectively.)

Sample Problem

A sample problem (SP3) is provided to demonstrate use of the new option for drainage from the unsaturated zone that is based on the conceptual model of coupled saturated-unsaturated flow developed by Mathias and Butler (2006). The problem uses data collected during a 7-day aquifer test completed in August 2001 at the well-instrumented field site at Canadian Forces Base Borden, near Barrie, Ontario, described in Bevan and others (2005). Over the past 25 years, the Borden site has been the focus of numerous investigations of saturated and unsaturated flow in response to pumping from an unconfined aquifer (Nwankwor and others, 1984, 1992; Akindunni and Gillham, 1992; Bevan and others, 2005; Endres and others, 2007). Data collection during the test included measuring drawdown at the pumped well and at more than 40 observation piezometers and monitoring of soil-moisture content profiles at six neutron moisture-probe access tubes (Bevan and others, 2005).

The unconfined aquifer at the Borden site is composed primarily of medium-grained sand of glacio-deltaic or glacio-fluvial origin (Bevan and others, 2005). Although the aquifer is locally heterogeneous because of discontinuous beds of fine-, medium-, and coarse-grained sand, overall it has a low degree of spatial variability and is considered to be homogeneous to flow at the scale of the aquifer test. The aquifer overlies a clayey silt confining aquifer that is assumed to be an impermeable barrier to flow at a depth of about 9 m below land surface. Depth to the water table prior to the test was about 2.75 m below land surface; therefore, the initial saturated thickness of the aquifer was about 6.25 m (fig. 6).

The pumped well at the site is screened over the bottom 3.65 m of the aquifer and has an internal radius of 0.065 m. The well was pumped at a constant discharge rate of 40 L/min (0.04 m³/min) during the 7-day test. Although drawdowns were monitored at all observation piezometers (Bevan and others, 2005), data collected from only seven of the piezometers are used in the sample problem. The radial distance of each piezometer from the pumped well, the center of the screened interval of each piezometer, and the radius of each piezometer are provided in table 1. Delayed drawdown response at all but one of the piezometers (P5) was simulated with WTAQ. In all cases, the length of the screened interval of each piezometer was specified to be 0.35 m, on the basis of information in Bevan and others (2005).

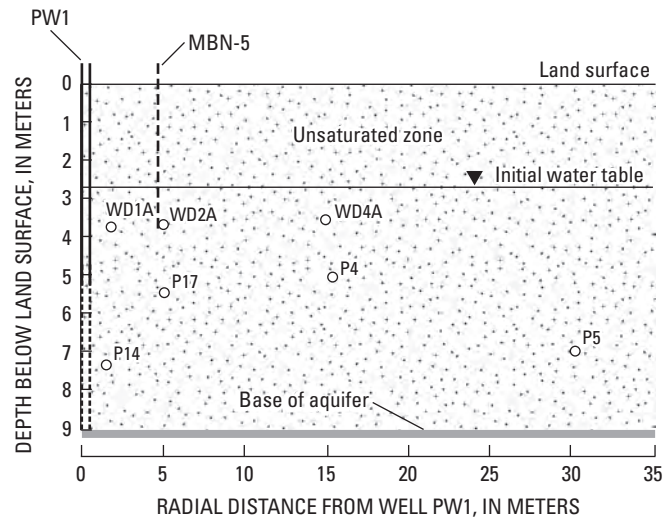


Figure 6. Vertical section of the aquifer at the field site showing the positions of the screened interval of the pumped well (identified as PW1), observation piezometers (shown as open circles, with identifiers), and neutron-access tube MBN-5. (Modified from Moench, 2008.)

Table 1. Location, depth, and radius of each observation piezometer for sample problem.

[All units are in meters]

Piezometer	Radial distance from center of pumped well	Depth below initial water table to center of piezometer well screen	Radius of piezometer
WD1A	1.51	0.94	0.025
WD2A	5.07	0.89	0.025
WD4A	15.05	0.84	0.025
P17	5.15	2.69	0.025
P4	15.36	2.32	0.0175
P14	1.51	4.57	0.025
P5	30.19	4.30	0.025

Data from the test have been used by Endres and others (2007) and Moench (2008) to estimate hydraulic properties of the aquifer. With the exception of the estimated value of specific storage, the values of the hydraulic properties estimated by the two researchers are similar but differ partly because Endres and others (2007) did not use the complete data set collected by Bevan (2002). Because the estimates made by Moench are based on the Mathias and Butler model, they are used in this sample problem. The estimates are (Moench, 2008, table 3a): b , 6.20 m; K_r , 4.10×10^{-3} m/min; K_z , 1.74×10^{-3} m/min; S_s , 3.76×10^{-5} m⁻¹; and S_y , 0.25 (see discussion following table 3a in Moench, 2008). Moench also estimated a value of the skin parameter for the pumped well (S_w) of 1.74. As part of the analysis, Moench (2008) assumed the unsaturated zone was infinitely thick. This assumption was based on soil-moisture measurements made at the field site that indicated the land surface was well above the transition zone that extends from the top of the capillary fringe at full moisture saturation to the water content at residual saturation (fig. 7A). Moench (2008) estimated a value of a_k equal to 5.0 m⁻¹ at the field site by visually fitting equation 1 to measurements of volumetric soil-moisture content above the initial water table made at neutron access tube MBN-5 (with ψ_s , the air-entry pressure equal, estimated to be -0.40 m). He also estimated a_k to be 31.7 m⁻¹ by use of the drawdown data available at the site and nonlinear parameter-estimation techniques.

The de Hoog numerical-inversion algorithm is required for simulating drawdown with the Mathias and Butler (2006) solution. The following program solution variables for the de Hoog algorithm, which are described in the appendix, were found to give adequate results: RERRNR = 1.0×10^{-10} , ERROR = 1.0×10^{-4} , NTMS = 30, NNN = 6, and METHOD = 3.

A data-input file for WTAQ version 2 was prepared on the basis of information given in the preceding discussion. Drawdown data measured at the piezometers are available in Michael Bevan’s thesis (Bevan, 2002) but are not listed here for proprietary reasons; if needed, the data can be requested from the University of Waterloo. Drawdowns calculated by WTAQ at the observation piezometers are plotted in figure 8 and, as would be expected, are consistent with the results shown in Moench (2008). The data-input and model-output files for this sample problem are shown in figures 9 and 10 (at back of report).

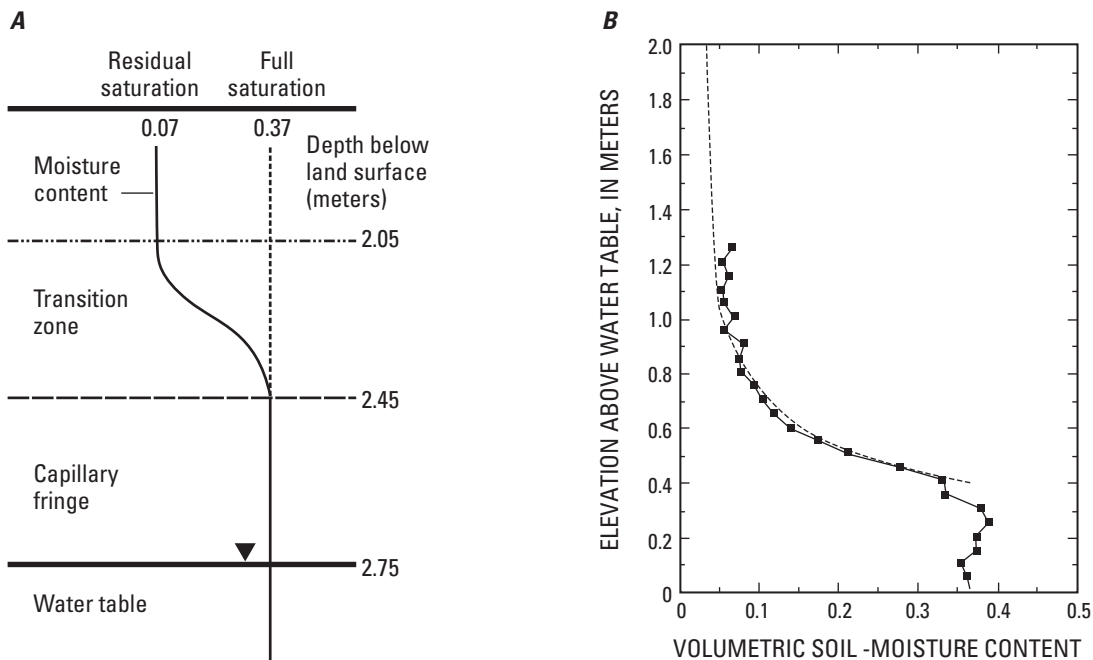


Figure 7. (A) Schematic diagram of the soil-moisture content profile at the field site prior to pumping (modified from Bevan and others, 2005) and (B) graph showing background volumetric soil-moisture content above initial water table for neutron-access tube MBN-5 (black squares) compared with an approximate fit to equation 1 with soil-moisture retention exponent (a_k) equal to 5 m⁻¹ (dashed line) (modified from Moench, 2008).

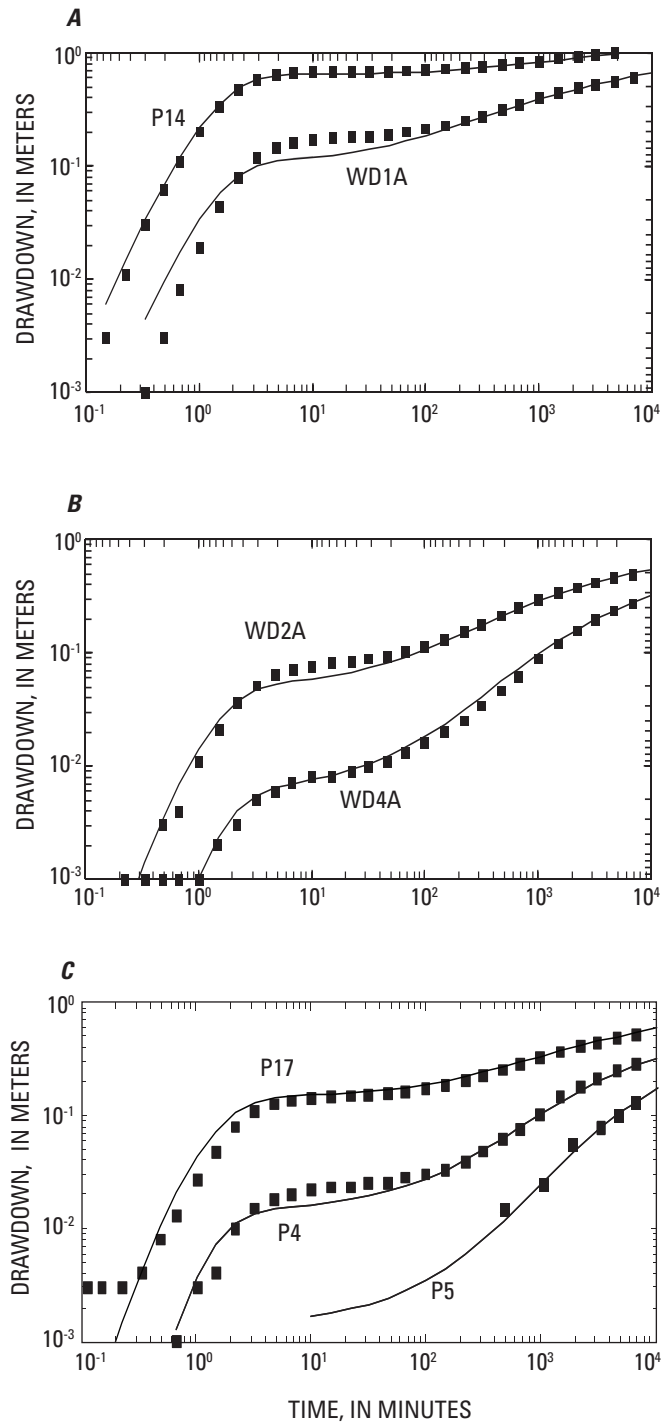


Figure 8. Measured and simulated drawdown at several observation piezometers at the field site: (A) piezometers P14 and WD1A; (B) WD2A and WD4A; and (C) P17, P4, and P5. Measured data are shown by squares; simulated results are shown by lines.

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Appendix 1— Data-Input Instructions for WTAQ Version 2

Description of New and Updated Program Options and Variables	16
Type-Curve Format.....	18
Dimensional Format.....	21
References Cited.....	26

Description of New and Updated Program Options and Variables

Program options and variables that are new to WTAQ version 2 or that have been updated for version 2 of the software are described below. The user is referred to the original WTAQ documentation for a description of additional program options and variables (Barlow and Moench, 1999, p. 8–12). The data-input format for version 2 is not the same as that for version 1; therefore, input files created for version 1 will not work with version 2. Nevertheless, the changes made for version 2 are relatively minor.

Program variables are shown in upper-case text.

Drainage from the unsaturated zone

WTAQ version 2 provides three approaches for simulating drainage from the unsaturated zone to water-table aquifers. The type of drainage that is simulated is specified by variable IDRA. The first two approaches are the same as those provided in version 1 of the code: IDRA is specified to be 0 for instantaneous drainage from the unsaturated zone (that is, Neuman-type drainage) and 1 for gradual drainage from the unsaturated zone (Boulton-type drainage). If gradual drainage is specified, values of the empirical drainage constants GAMMA(I) or ALPHA(I) also must be specified, as described by Barlow and Moench (1999, p. 9). The third approach explicitly accounts for characteristics of the unsaturated zone and is based on the model of Mathias and Butler (2006). To use this approach, IDRA is set to 2, and the following four variables also must be specified: ACC, the soil-moisture retention exponent (a_c ; units of inverse length); AKK, the relative hydraulic-conductivity exponent (a_k ; units of inverse length); AMM, the initial unsaturated-zone thickness above the capillary fringe (m ; units of length); and AXMM, the unsaturated-zone thickness above which an assumption of an infinitely thick unsaturated-zone thickness is assumed (units of length). The value specified for AKK must be greater than or equal to that specified for ACC; otherwise, an error message will be written to the program output file and the program will terminate. As described in the “Evaluation of Revised Program” section of this report, the solution of Mathias and Butler involves less computation to evaluate if the unsaturated zone can be assumed to be of infinite thickness. To simulate an infinitely thick unsaturated zone, the user should specify a value of AMM that is greater than or equal to the value of AXMM; for a finite-thickness unsaturated zone, set AXMM greater than AMM.

Numerical-Inversion Methods and Program Solution Variables

Two methods for numerical inversion of the Laplace-transform solutions are provided with WTAQ. The user must select either the Stehfest algorithm (Stehfest, 1970) (by setting ISOLN = 1) or de Hoog algorithm (de Hoog and others, 1982) (ISOLN = 2). In WTAQ, the Stehfest algorithm must be used for simulation of confined aquifers. Either the Stehfest or de Hoog algorithms can be used for water-table aquifers with either instantaneous (IDRA = 0) or gradual (IDRA = 1) drainage from the unsaturated zone. The de Hoog algorithm must be used for water-table aquifers with drainage that is based on the Mathias and Butler (2006) model (IDRA = 2).

Each of the two numerical-inversion techniques requires specification of five program-solution variables. Those required for the Stehfest algorithm are described in Barlow and Moench (1999, p. 10). The five variables necessary for the de Hoog algorithm are RERRNR, ERROR, NTMS, NNN, and METHOD. (Variables RERRNR and NTMS also are used with the Stehfest algorithm.) Suggested values are provided for each of these variables, and the user should not have to make changes to them. Variable RERRNR is the relative error for Newton-Raphson iteration used in the finite summations of the analytical solutions for drawdown in water-table aquifers developed by Moench (1997). Smaller values of RERRNR will increase solution precision and time. A value of 1.0D-10 is suggested. If the value of RERRNR is exceeded after 100 Newton-Raphson iterations, a message is printed to the result file, and the program is stopped. Should this happen, the user can increase the value of RERRNR and re-run the simulation. Variable NTMS is a factor used to determine the number of terms in the same finite summations. Suggested values are 20 or 30. The user should ensure that a sufficient number of terms are being used in the summations by making multiple runs in which NTMS is increased from one simulation to the next (for example, doubled) and should continue until simulation results do not vary substantially when NTMS is increased.

Variable ERROR is the relative error sought for the accuracy of the numerical inversion. A value of 1.0D-04 is recommended. Variable METHOD is an integer variable that determines the method that will be used to accelerate convergence of the summation of the Fourier series of the approximation to the inverse Laplace transform. Three options (METHOD = 1, 2, or 3) are provided in WTAQ; the options are based on subroutine LAPADC, which was developed by John Knight (School of Mathematical Sciences, Queensland University of Technology, Brisbane, Australia) and is implemented in WTAQ. Only METHOD = 3 has been tested with WTAQ version 2 and has been found to be satisfactory. The user can consult de Hoog and others (1982) and subroutine LAPADC for detailed discussion of the options if needed. Variable NNN is the number of terms used in the summation of the Fourier series and affects truncation and roundoff errors of the summation. NNN must

be an even number greater than or equal to 2. As NNN is increased, truncation error decreases, but roundoff error increases. A value of 6 is suggested, but the user can ensure that a sufficient number of terms are being used by making multiple runs in which NNN is increased from one simulation to the next and confirming that that simulation results do not vary substantially when NNN is changed. Under unusual conditions, computations of the analytical expression for drainage from the unsaturated zone (equation 42 in Mathias and Butler, 2006) may be unreliable. This can occur for small values of time or small values of dimensionless parameter β :

$$\beta = \left(\frac{K_z}{K_r} \right) \left(\frac{r}{b} \right)^2,$$

where K_z/K_r is the ratio of vertical to radial hydraulic conductivity, r is the radial distance of the well or observation piezometer from the center of the pumping well, and b is the initial saturated thickness of the aquifer. To ensure that all of the calculations are correct, the user should check that calculated drawdowns increase smoothly with time, and that there are no oscillations in the calculated drawdowns from one time to the next.

Additional Updates for Version 2

A number of additional changes were made for version 2 to improve program operation. These changes are described here. First, although not a change from the first version of the code, to avoid confusion it should be noted that the vertical positions of the screened interval of the pumped well (variables ZPD and ZPL) and observation wells (Z1 and Z2), as well as the vertical position of the center of each piezometer (ZP), are defined with reference to the top of the aquifer or initial water table, as opposed to the height above the base of the aquifer (z) as used in figures 1 and 2.

Calculations of drawdown at the pumped well usually require more computations than for distant points of observation. To avoid such computations when not needed, the variable IPUMP has been added to allow users to skip calculations of drawdown at the pumped well (IPUMP = 0). Computation times also can be large for small values of β (Moench, 1996, p. 595).

WTAQ users must now specify the name of each observation well or piezometer by use of variable OBNAME, which is a character variable that must be less than or equal to 10 characters in length.

Version 1 of WTAQ provided an option for simulation of delayed drawdown response at observation wells due to the effects of stored water within the wells but did not allow simulation of these effects at observation piezometers. This restriction has been removed for version 2. If delayed drawdown response at observation piezometers is specified (that is, variable IDPR is set to 1), the user must also specify a value of WDP (delayed-response factor) when using the type-curve format of WTAQ or values of RP (inside radius of the piezometer) and XLL (length of the screened interval of the piezometer) when using the dimensional format (see additional details about these variables in Barlow and Moench, 1999, p. 12).

Type-Curve Format

Line-by-line instructions for creating a data-input file for the type-curve format follow. Variable names used in the input file and computer program are shown in upper-case text. As noted previously, the data-input format for version 2 of WTAQ is not the same as that for version 1; therefore, input files created for version 1 of the software will not work with version 2.

Part I: Aquifer Characteristics and General Information

Line 1: TITLE

TITLE—Title of simulation; up to 70 characters in length. Leave this line blank if no title is specified.

Line 2: FORMAT

FORMAT—Analysis format. Enter TYPE CURVE.

Line 3: AQTYPE

AQTYPE—Type of aquifer being simulated. Two options are provided:

AQTYPE = CONFINED or

AQTYPE = WATER TABLE

Line 4: BB XKD SIGMA

BB—Thickness or saturated thickness of aquifer at beginning of simulation, units of length.

XKD—Ratio of vertical to horizontal hydraulic conductivity of aquifer, dimensionless.

SIGMA—Ratio of storativity to specific yield of aquifer, dimensionless. Enter 0.0D0 if AQTYPE = CONFINED.

Line 5: IDRA NGAMMA

IDRA—Type of drainage at water table. Enter 0 if AQTYPE = CONFINED. Three options are provided:

IDRA = 0: Instantaneous drainage.

IDRA = 1: Gradual drainage.

IDRA = 2: Drainage with unsaturated-zone characterization.

NGAMMA—Number of dimensionless drainage constants. Enter 0 if IDRA = 0 or 2. NGAMMA must be less than or equal to 5.

Line 6: Data input for line 6 depends on the value of IDRA.

If IDRA = 0, enter: 1.0D09

If IDRA = 1, enter: GAMMA(I)

GAMMA(I)—A single row of NGAMMA dimensionless drainage constants. Successive values must be separated by at least 1 space. Maximum of 5 values is allowed.

If IDRA = 2, enter: ACC AKK AMM AXMM

ACC—Soil-moisture retention exponent, in units of inverse length.

AKK—Relative hydraulic-conductivity exponent, in units of inverse length. The value specified must be greater than or equal to that specified for ACC.

AMM—Initial unsaturated-zone thickness above the capillary fringe, in units of length.

AXMM—The unsaturated-zone thickness above the capillary fringe above which an assumption of an infinitely thick unsaturated-zone thickness is assumed, in units of length.

Line 7: TDLAST NLC NOX

TDLAST—Largest value of dimensionless time.

NLC—Number of logarithmic cycles on the time scale for which drawdown will be calculated.

NOX—Number of equally spaced times per logarithmic cycle for which drawdown will be calculated.

Line 8: ISOLN

ISOLN—Numerical-inversion solution type:

ISOLN = 1: Solution by the Stehfest algorithm (must use this option for confined aquifers).

ISOLN = 2: Solution by the de Hoog algorithm (must use this option for IDRA = 2).

Line 9: Data input for this line depends on the value of ISOLN.

If ISOLN = 1 (Stehfest algorithm), enter: RERRNR RERRSUM NMAX NTMS NS

RERRNR—Relative error for Newton-Raphson iteration and finite summations of drawdown for water-table aquifers.

A value of 1.0D-10 is suggested. Enter 0.0D0 for AQTYPE = CONFINED.

RERRSUM—Relative error for finite summations of drawdown for confined aquifers. Suggested value is 1.0D-07 to 1.0D-08.

Enter 0.0D0 if AQTYPE = WATER TABLE.

NMAX—Maximum number of terms permitted in the finite summations of drawdown for confined aquifers.

Suggested value is 200. Enter 0 if AQTYPE = WATER TABLE.

NTMS—Factor used to determine number of terms in the finite summations for drawdown for water-table aquifers.

Suggested values are 20 or 30. Enter 0 if AQTYPE = CONFINED.

NS—Number of terms used in the Stehfest algorithm. This must be an even integer, the value of which depends on computer precision. If the computer holds 16 significant figures in double precision, let NS = 6 to 12. A value of 8 is recommended.

If ISOLN = 2 (de Hoog algorithm), enter: RERRNR ERROR NTMS NNN METHOD

RERRNR—Relative error for Newton-Raphson iteration and finite summations of drawdown for water-table aquifers.

A value of 1.0D-10 is suggested.

ERROR—Relative error sought for the accuracy of the numerical inversion. A value of 1.0D-04 is suggested.

NTMS—Factor used to determine number of terms in the finite summation for drawdown for water-table aquifers.

Suggested values are 20 or 30.

NNN—Number of terms used in the summation of the Fourier series of the approximation to the inverse Laplace transform.

A value of 6 is suggested.

METHOD—Indicates which method will be used to accelerate convergence of the Fourier series. Options are 1, 2, or 3.

Only METHOD = 3 has been tested and was found to be satisfactory. Users can consult de Hoog and others (1982) and John Knight's subroutine LAPADC for additional details if needed.

Part II: Pumped-Well Information

Line 10: IPWS IPWD IPUMP

IPWS—Type of pumped well:

IPWS = 0: Partially penetrating pumped well.

IPWS = 1: Fully penetrating pumped well.

IPWD—Type of diameter of pumped well:

IPWD = 0: Infinitesimal diameter (line-source theory).

IPWD = 1: Finite diameter.

IPUMP—Option to suppress calculations of drawdown at pumped well:

IPUMP = 0: Drawdown is not calculated at pumped well.

IPUMP = 1: Drawdown is calculated at pumped well.

Line 11: RW ZPD ZPL WD SW

RW—Radius of pumped well screen, in units of length.

ZPD—Depth below top of aquifer or initial water table to the top of the screened interval of the pumped well, in units of length.

ZPL—Depth below top of aquifer or initial water table to the bottom of the screened interval of the pumped well, in units of length.

WD—Well-bore storage parameter, dimensionless. Enter 0.0D0 if IPWD = 0.

SW—Well-bore skin parameter, dimensionless.

Part III: Observation-Well Information

Line 12: NOBWC

NOBWC—Number of observation wells or piezometers for which drawdown curves will be calculated. Must be less than or equal to 25.

Lines 13 and 14: Repeat lines 13 and 14 for each of the NOBWC observation wells or piezometers.

Line 13: OBNAME IOWS IDPR

OBNAME—Name of observation well or piezometer; up to 10 characters in length.

IOWS—Type of observation well or piezometer:

IOWS = 0: Partially penetrating observation well.

IOWS = 1: Fully penetrating observation well.

IOWS = 2: Observation piezometer.

IDPR—Options for delayed response of observation well.

IDPR = 0: No delayed response.

IDPR = 1: Delayed response.

Line 14: R Z1 Z2 ZP WDP

R—Radial distance from axis of pumped well to observation well or piezometer, in units of length.

Z1—Depth below top of aquifer or initial water table to the top of screened interval of observation well, in units of length. Use for IOWS = 0 or 1. Enter 0.0D0 if IOWS = 2.

Z2— Depth below top of aquifer or initial water table to the bottom of screened interval of observation well, in units of length. Use for IOWS = 0 or 1. Enter 0.0D0 if IOWS = 2.

ZP—Depth below top of aquifer or initial water table to center of piezometer, in units of length. Use for IOWS = 2. Enter 0.0D0 if IOWS = 0 or 1.

WDP— Delayed response factor, dimensionless. Enter 0.0D0 if IDPR = 0.

Dimensional Format

Line-by-line instructions for creating a data-input file for the dimensional format follow. Variable names used in the input file and computer program are shown in upper-case text. As noted previously, the data-input format for version 2 of WTAQ is not the same as that for version 1; therefore, input files created for version 1 of the software will not work with version 2.

Part I: Aquifer Characteristics and General Information

Line 1: TITLE

TITLE—Title of simulation; up to 70 characters in length. Leave this line blank if no title is specified.

Line 2: FORMAT

FORMAT—Analysis format. Enter DIMENSIONAL.

Line 3: AQTYPE

AQTYPE—Type of aquifer being simulated. Two options are provided:

AQTYPE = CONFINED or

AQTYPE = WATER TABLE

Line 4: BB HKR HKZ SS SY

BB—Thickness or saturated thickness of aquifer at beginning of simulation, in units of length.

HKR—Horizontal hydraulic conductivity of aquifer, in units of length per time.

HKZ—Vertical hydraulic conductivity of aquifer, in units of length per time.

SS—Specific storage of aquifer, in units of inverse length.

SY—Specific yield of aquifer, dimensionless. Enter 0.0D0 if AQTYPE = CONFINED.

Line 5: IDRA NALPHA

IDRA—Type of drainage at water table. Enter 0 if AQTYPE = CONFINED. Three options are provided:

IDRA = 0: Instantaneous drainage.

IDRA = 1: Gradual drainage.

IDRA = 2: Drainage with unsaturated-zone characterization.

NALPHA—Number of drainage constants. Enter 0 if IDRA = 0 or 2. NALPHA must be less than or equal to 5.

22 WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers

Line 6: Data input for line 6 depends on the value of IDRA.

If IDRA = 0, enter: 1.0D09

If IDRA = 1, enter: ALPHA(I)

ALPHA(I)—A single row of NALPHA drainage constants, in units of inverse time. Successive values must be separated by at least 1 space. Maximum of 5 values is allowed.

If IDRA = 2, enter: ACC AKK AMM AXMM

ACC—Soil-moisture retention exponent, in units of inverse length.

AKK—Relative hydraulic-conductivity exponent, in units of inverse length. The value specified must be greater than or equal to that specified for ACC.

AMM—Initial unsaturated-zone thickness above the capillary fringe, in units of length.

AXMM—The unsaturated-zone thickness above the capillary fringe above which an assumption of an infinitely thick unsaturated-zone thickness is assumed, in units of length.

Line 7: ITS IMEAS

ITS—Time specification:

ITS = 0: Log-cycle time steps (use to generate theoretical curves).

ITS = 1: User-specified times.

IMEAS—Specification of measured drawdown data. Enter 0 if ITS = 0. Options for ITS = 1:

IMEAS = 0: Measured drawdown data not specified for each time.

IMEAS = 1: Measured drawdown data specified for each time.

Line 8: TLAST NLC NOX

TLAST—Largest value of time. Enter 0.0D0 if ITS = 1.

NLC—Number of logarithmic cycles on the time scale for which drawdown will be calculated. Enter 0 if ITS = 1.

NOX—Number of equally spaced times per logarithmic cycle for which drawdown will be calculated. Enter 0 if ITS = 1.

Line 9: ISOLN

ISOLN—Numerical-inversion solution type:

ISOLN = 1: Solution by the Stehfest algorithm (must use this option for confined aquifers).

ISOLN = 2: Solution by the de Hoog algorithm (must use this option for IDRA = 2).

Line 10: Data input for line 10 depends on the value of ISOLN.

If ISOLN = 1 (Stehfest algorithm), enter: RERRNR RERRSUM NMAX NTMS NS

RERRNR—Relative error for Newton-Raphson iteration and finite summations of drawdown for water-table aquifers. A value of 1.0D-10 is suggested. Enter 0.0D0 for AQTYPE = CONFINED.

RERRSUM—Relative error for finite summations of drawdown for confined aquifers. Suggested value is 1.0D-07 to 1.0D-08. Enter 0.0D0 if AQTYPE = WATER TABLE.

NMAX—Maximum number of terms permitted in the finite summations of drawdown for confined aquifers. Suggested value is 200. Enter 0 if AQTYPE = WATER TABLE.

NTMS—Factor used to determine number of terms in the finite summations for drawdown for water-table aquifers. Suggested values are 20 or 30. Enter 0 if AQTYPE = CONFINED.

NS—Number of terms used in the Stehfest algorithm. This must be an even integer, the value of which depends upon computer precision. If the computer holds 16 significant figures in double precision, let NS = 6 to 12. A value of 8 is recommended.

If ISOLN = 2 (de Hoog algorithm), enter: RERRNR ERROR NTMS NNN METHOD

RERRNR—Relative error for Newton-Raphson iteration and finite summations of drawdown for water-table aquifers. A value of 1.0D-10 is suggested.

ERROR—Relative error sought for the accuracy of the numerical inversion. A value of 1.0D-04 is suggested.

NTMS—Factor used to determine number of terms in the finite summation for drawdown for water-table aquifers. Suggested values are 20 or 30.

NNN—Number of terms used in the summation of the Fourier series of the approximation to the inverse Laplace transform. A value of 6 is suggested.

METHOD—Indicates which method will be used to accelerate convergence of the Fourier series. Options are 1, 2, or 3. Only METHOD = 3 has been tested and was found to be satisfactory. Users can consult de Hoog and others (1982) and John Knight's subroutine LAPADC for additional details if needed.

Part II: Pumped-Well Information

Line 11: IPWS IPWD IPUMP

IPWS—Type of pumped well:

IPWS = 0: Partially penetrating pumped well.

IPWS = 1: Fully penetrating pumped well.

IPWD—Type of diameter of pumped well:

IPWD = 0: Infinitesimal diameter (line-source theory).

IPWD = 1: Finite diameter.

IPUMP—Option to suppress calculations of drawdown at pumped well:

IPUMP = 0: Drawdown is not calculated at pumped well.

IPUMP = 1: Drawdown is calculated at pumped well.

24 WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers

Line 12: QQ RW RC ZPD ZPL SW

QQ—Pumping rate of well, in units of cubic length per time.

RW—Radius of pumped well screen, in units of length.

RC—Inside radius of pumped well in the interval where water levels are changing during pumping, in units of length.
Enter 0.0D0 if $IPWD = 0$.

ZPD—Depth below top of aquifer or initial water table to the top of the screened interval of the pumped well, in units of length.

ZPL—Depth below top of aquifer or initial water table to the bottom of the screened interval of the pumped well,
in units of length.

SW—Well-bore skin parameter, dimensionless.

Lines 13 and 14: Information on user-specified times for which drawdowns will be calculated for the pumped well. Skip to line 15 if $ITS = 0$.

Line 13: NTSPW IRUN

NTSPW—Number of user-specified times for which drawdown at the pumped well will be calculated. If $NTSPW = 0$, no drawdowns are calculated for the pumped well.

IRUN—Option to suppress drawdown calculations for the pumped well. Allows user to specify time-drawdown data (Line 14), but those data are ignored during the simulation. Options are:

IRUN = 0: Drawdowns not calculated.

IRUN = 1: Drawdowns calculated.

Line 14: Time and measured drawdown data for pumped well. Repeat this line of input data NTSPW times
(if $NTSPW = 0$, skip to line 15): TIMEPW(I) XMEASPW(I)

TIMEPW(I)—Time I.

XMEASPW(I)—Measured drawdown at pumped well for time I, in units of length. Leave blank if $IMEAS = 0$.

Part III: Observation-Well Information

Line 15: NOBWC

NOBWC—Number of observation wells or piezometers for which drawdown curves will be calculated. Must be less than or equal to 25.

Lines 16 through 19: Repeat lines 16 through 19 for each of the NOBWC observation wells or piezometers.

Line 16: OBNAME IOWS IDPR

OBNAME—Name of observation well or piezometer; up to 10 characters in length.

IOWS—Type of observation well or piezometer:

IOWS = 0: Partially penetrating observation well.

IOWS = 1: Fully penetrating observation well.

IOWS = 2: Observation piezometer.

IDPR—Options for delayed response of observation well.

IDPR = 0: No delayed response.

IDPR = 1: Delayed response.

Line 17: R Z1 Z2 ZP RP XLL

R—Radial distance from axis of pumped well to observation well or piezometer, in units of length.

Z1—Depth below top of aquifer or initial water table to the top of screened interval of observation well, in units of length.

Use for IOWS = 0 or 1. Enter 0.0D0 if IOWS = 2.

Z2—Depth below top of aquifer or initial water table to the bottom of screened interval of observation well, in units of length.

Use for IOWS = 0 or 1. Enter 0.0D0 if IOWS = 2.

ZP—Depth below top of aquifer or initial water table to center of piezometer, in units of length. Use for IOWS = 2.

Enter 0.0D0 if IOWS = 0 or 1.

RP—Inside radius of the observation well (or piezometer) standpipe in the interval over which water levels are changing during pumping, in units of length. Enter 0.0D0 if IDPR = 0.

XLL—Length of screened interval of observation well or piezometer, in units of length. Enter 0.0D0 if IDPR = 0.

Lines 18 and 19: Information on user-specified times for which drawdowns will be calculated for the observation well or piezometer. Skip these lines if ITS = 0.

Line 18: NTSOB IRUN

NTSOB—Number of user-specified times for which drawdown at the observation well or piezometer will be calculated.

If NTSOB = 0, no drawdowns are calculated for the observation well or piezometer.

IRUN—Option to suppress drawdown calculations for the observation well or piezometer. Allows user to specify time-drawdown data (Line 19), but those data are ignored during the simulation. Options are:

IRUN = 0: Drawdowns not calculated.

IRUN = 1: Drawdowns calculated.

Line 19: Time and measured drawdown data for observation well or piezometer. Repeat this line of input data NTSOB times (if NTSOB = 0, skip to next observation point): TIMEOB(I) XMEASOB(I)

TIMEOB(I)—Time I.

XMEASOB(I)—Measured drawdown at observation well or piezometer for time I, in units of length.

Leave blank if IMEAS = 0.

References Cited

- Barlow, P.M., and Moench, A.F., 1999, WTAQ—A computer program for calculating drawdowns and estimating hydraulic properties for confined and water-table aquifers: U.S. Geological Survey Water-Resources Investigations Report 99-4225, 74 p.
- de Hoog, F.R., Knight, J.H., and Stokes, A.N., 1982, An improved method for numerical inversion of Laplace transforms: SIAM Journal of Scientific Computing, v. 3, p. 357–366.
- Mathias, S.A., and Butler, A.P., 2006, Linearized Richards' equation approach to pumping test analysis in compressible aquifers: Water Resources Research, v. 42, W06408, doi:10.1029/2005WR004680, 10 p.
- Moench, A.F., 1996, Flow to a well in a water-table aquifer: An improved Laplace transform solution: Ground Water, v. 34, no. 4, p. 593–596.
- Moench, A.F. 1997, Flow to a well of finite diameter in a homogeneous, anisotropic water table aquifer: Water Resources Research, v. 33, no. 6, p. 1397–1407.
- Stehfest, Harald, 1970, Numerical inversion of Laplace transforms: Communications of the Association for Computing Machinery (ACM), v. 13, no. 1, p. 47–49.

Figures 9 and 10

Figure 9. Data-input file for sample problem.

```

Sample problem 3, with Borden site conditions (minutes, meters).
DIMENSIONAL                                FORMAT
WATER TABLE                               AQTYPE
6.20D0 4.10E-03 1.74E-03 3.76E-05 0.25     BB HKR HKZ SS SY
2 0                                          IDRA NALPHA
5.0D0 31.7D0 100.D0 10.D0                 ACC AKK AMM AXMM
1 0                                          ITS IMEAS
0.0D0 0 0                                   TLAST NLC NOX
2                                          ISOLN (de Hoog)
1.0D-10 1.D-4 30 6 3                       RERRNR ERROR NTMS NNN METHOD
0 1 1                                       IPWS IPWD IPUMP
40.d-03 0.065d0 0.065d0 2.55D0 6.20D0 1.74 QQ RW RC ZPD ZPL SW
33 1                                         NTSPW IRUN
.0373                                       TIMEPW(I)
.0747
.1120
.1493
.2240
.3360
.4853
0.6720
1.0065
1.5270
2.2273
3.2755
4.8443
6.7958
10.1120
15.0737
22.4978
31.7328
47.4245
66.9433
100.1085
149.7313
223.9793
316.3345
473.2577
668.4498
1000.1053
1496.3410
2238.8263
3162.3827
4697.0700
6857.0700
10457.0700

```

7
 WD1A 2 1
 1.51 0.0 0.0 0.94 0.025 0.35
 28 1
 .3360
 .4853
 0.6720
 1.0065
 1.5270
 2.2273
 3.2755
 4.8443
 6.7958
 10.1120
 15.0737
 22.4978
 31.7328
 47.4245
 66.9433
 100.1085
 149.7313
 223.9793
 316.3345
 473.2577
 668.4498
 1000.1053
 1496.3410
 2238.8263
 3162.3827
 4697.0700
 6857.0700
 10457.0700

NOBWC
 OBNAME IOWS IDPR
 WD1A: R Z1 Z2 ZP RP XXL
 NTSOB IRUN
 TIMEOB (I)

WD2A 2 1
 5.07 0.0 0.0 0.89 0.025 0.35
 29 1
 .2240
 .3360
 .4853
 0.6720
 1.0065
 1.5270
 2.2273
 3.2755
 4.8443
 6.7958
 10.1120
 15.0737
 22.4978
 31.7328
 47.4245
 66.9433
 100.1085
 149.7313
 223.9793
 316.3345
 473.2577
 668.4498

WD2A
 NTSOB IRUN
 TIMEOB (I)

30 WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers

1000.1053
1496.3410
2238.8263
3162.3827
4697.0700
6857.0700
10457.0700

WD4A 2 1

15.05 0.0 0.0 0.84 0.025 0.35

27 1

.4875

0.6750

1.0252

1.4930

2.1933

3.2415

4.8103

6.7618

10.0780

15.0397

22.4638

31.6988

47.3905

66.9093

100.0745

149.6973

223.9453

316.3005

473.2237

668.4158

1000.0713

1496.3070

2238.7923

3162.3487

4697.0360

6857.0360

10457.0360

P17 2 1

5.15 0.0 0.0 2.69 0.025 0.35

33 1

.0375

.0750

.1125

.1500

.2250

.3375

.4875

0.6750

1.0252

1.4930

2.1933

3.2415

4.8103

6.7618

10.0780

15.0397

22.4638

WD4A
NTSOB IRUN
TIMEOB (I)

P17
NTSOB IRUN
TIMEOB (I)

31.6988
 47.3905
 66.9093
 100.0745
 149.6973
 223.9453
 316.3005
 473.2237
 668.4158
 1000.0713
 1496.3070
 2238.7923
 3162.3487
 4697.0360
 6857.0360
 10457.0360

P4 2 1
 15.36 0.0 0.0 2.32 0.0175 0.35
 26 1

P4
 NTSOB IRUN
 TIMEOB (I)

0.6750
 1.0252
 1.4930
 2.1933
 3.2415
 4.8103
 6.7618
 10.0780
 15.0397
 22.4638
 31.6988
 47.3905
 66.9093
 100.0745
 149.6973
 223.9453
 316.3005
 473.2237
 668.4158
 1000.0713
 1496.3070
 2238.7923
 3162.3487
 4697.0360
 6857.0360
 10457.0360

P14 2 1
 1.51 0.0 0.0 4.57 0.025 0.35
 30 1

P14
 NTSOB IRUN
 TIMEOB (I)

.1493
 .2240
 .3360
 .4853
 0.6720
 1.0065
 1.5270
 2.2273
 3.2755

32 WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers

4.8443
6.7958
10.1120
15.0737
22.4978
31.7328
47.4245
66.9433
100.1085
149.7313
223.9793
316.3345
473.2577
668.4498
1000.1053
1496.3410
2238.8263
3162.3827
4697.0700
6857.0700
10457.0700
P5 2 0
30.19 0.0 0.0 4.30 0.025 0.35
7 1
480.0000
1080.0000
1950.0000
3420.0000
4920.0000
6750.0000
10530.0000

P5
NTSOB IRUN
TIMEOB (I)

Figure 10. Output-results file for sample problem.

```

*****
*
*          ****  U.S. GEOLOGICAL SURVEY  ****
*
*          ****  WTAQ: PROGRAM OUTPUT  ****
*
*  COMPUTER PROGRAM FOR CALCULATING DRAWDOWN
*
*  IN A CONFINED OR WATER-TABLE AQUIFER WITH
*
*  AXIAL-SYMMETRIC FLOW TO A FINITE- OR
*
*  INFINITESIMAL-DIAMETER PUMPED WELL
*
*          VERSION 2.0, JANUARY 2011
*
*****

```

Sample problem 3, with Borden site conditions (minutes, meters).

DIMENSIONAL FORMAT

WATER-TABLE AQUIFER

*** AQUIFER HYDRAULIC PROPERTIES ***

```

SATURATED THICKNESS (BB):          0.620D+01 (units of length)
HORIZONTAL HYDRAULIC
CONDUCTIVITY (HKR):                0.410D-02 (units of length per time)
VERTICAL HYDRAULIC
CONDUCTIVITY (HKZ):                0.174D-02 (units of length per time)
RATIO OF VERTICAL TO HORIZONTAL
HYDRAULIC CONDUCTIVITY (XKD):      0.424D+00 (dimensionless)
CALCULATED TRANSMISSIVITY:         0.254D-01 (units of length squared
                                         per time)
SPECIFIC STORAGE (SS):             0.376D-04 (units of inverse length)
SPECIFIC YIELD (SY):               0.250D+00 (dimensionless)
CALCULATED STORATIVITY:            0.233D-03 (dimensionless)
RATIO OF STORATIVITY TO
SPECIFIC YIELD (SIGMA):            0.932D-03 (dimensionless)
DRAINAGE AT WATER TABLE (IDRA):   2 (unsaturated-zone characteristics)

```

SPECIFIED UNSATURATED-ZONE PARAMETERS:

```

MOISTURE RETENTION EXPONENT (ACC):  0.500D+01 (units of inverse length)
RELATIVE PERMEABILITY EXPONENT (AKK): 0.317D+02 (units of inverse length)
INITIAL UNSAT. ZONE THICKNESS (AMM): 0.100D+03 (units of length)
MAX. UNSAT. ZONE THICKNESS (AXMM):   0.100D+02 (units of length)

```

DIMENSIONLESS UNSATURATED-ZONE PARAMETERS CALCULATED BY WTAQ:

```

MOISTURE RETENTION EXPONENT (ADC):   0.310D+02
RELATIVE PERMEABILITY EXPONENT (ADK): 0.197D+03

```

34 WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers

INITIAL UNSAT. ZONE THICKNESS (ADM): 0.161D+02
 MAX. UNSAT. ZONE THICKNESS (XMAXDM): 0.161D+01

INITIAL SATURATED THICKNESS OF UNSATURATED ZONE IS GREATER THAN THE MAXIMUM UNSATURATED-ZONE THICKNESS SPECIFIED: SATURATED-ZONE THICKNESS ASSUMED INFINITE.

*** PROGRAM SOLUTION VARIABLES ***

USER-SPECIFIED TIMES; MEASURED DRAWDOWN DATA NOT SPECIFIED

SOLUTION USING DE HOOG ALGORITHM:

RERRNR	ERROR	NTMS	NNN	METHOD
0.100D-09	0.100D-03	30	6	3

*** PUMPED-WELL CHARACTERISTICS AND CALCULATED DRAWDOWN ***

WELL-DIAMETER TYPE (IPWD): 1 (finite diameter)
 SCREENED INTERVAL (IPWS): 0 (partially penetrating)
 PUMPING RATE OF WELL (QQ): 0.400D-01 (cubic length per time)

WELL RADIUS	SCREENED INTERVAL		WELL BORE STORAGE	WELL BORE SKIN
	ZPD	ZPL		
0.65D-01	0.25D+01	0.62D+01	0.36D+04	0.17D+01

BETAW = 0.466D-04

TIME	CALCULATED DRAWDOWN
0.3730D-01	0.1094D+00
0.7470D-01	0.2138D+00
0.1120D+00	0.3133D+00
0.1493D+00	0.4084D+00
0.2240D+00	0.5867D+00
0.3360D+00	0.8267D+00
0.4853D+00	0.1102D+01
0.6720D+00	0.1388D+01
0.1006D+01	0.1771D+01
0.1527D+01	0.2144D+01
0.2227D+01	0.2401D+01
0.3276D+01	0.2552D+01
0.4844D+01	0.2609D+01
0.6796D+01	0.2621D+01
0.1011D+02	0.2624D+01
0.1507D+02	0.2627D+01
0.2250D+02	0.2631D+01
0.3173D+02	0.2636D+01

0.4742D+02	0.2643D+01
0.6694D+02	0.2650D+01
0.1001D+03	0.2662D+01
0.1497D+03	0.2677D+01
0.2240D+03	0.2696D+01
0.3163D+03	0.2716D+01
0.4733D+03	0.2743D+01
0.6684D+03	0.2771D+01
0.1000D+04	0.2806D+01
0.1496D+04	0.2845D+01
0.2239D+04	0.2887D+01
0.3162D+04	0.2925D+01
0.4697D+04	0.2969D+01
0.6857D+04	0.3013D+01
0.1046D+05	0.3062D+01

*** OBSERVATION-WELL CHARACTERISTICS AND CALCULATED DRAWDOWN ***

OBSERVATION WELL OR PIEZOMETER: WD1A

OBSERVATION PIEZOMETER

DISTANCE FROM CENTER OF PUMPED WELL	ZP	DELAYED RESPONSE FACTOR
0.151D+01	0.940D+00	0.173D+05

DRAWDOWN CALCULATED FOR BETA = 0.252D-01

TIME	CALCULATED DRAWDOWN
0.3360D+00	0.4394D-02
0.4853D+00	0.9478D-02
0.6720D+00	0.1747D-01
0.1006D+01	0.3360D-01
0.1527D+01	0.5725D-01
0.2227D+01	0.8052D-01
0.3276D+01	0.9923D-01
0.4844D+01	0.1097D+00
0.6796D+01	0.1143D+00
0.1011D+02	0.1189D+00
0.1507D+02	0.1246D+00
0.2250D+02	0.1321D+00
0.3173D+02	0.1403D+00
0.4742D+02	0.1527D+00
0.6694D+02	0.1662D+00
0.1001D+03	0.1858D+00
0.1497D+03	0.2104D+00

0.2240D+03	0.2400D+00
0.3163D+03	0.2694D+00
0.4733D+03	0.3079D+00
0.6684D+03	0.3439D+00
0.1000D+04	0.3883D+00
0.1496D+04	0.4345D+00
0.2239D+04	0.4817D+00
0.3162D+04	0.5226D+00
0.4697D+04	0.5698D+00
0.6857D+04	0.6150D+00
0.1046D+05	0.6657D+00

OBSERVATION WELL OR PIEZOMETER: WD2A

OBSERVATION PIEZOMETER

DISTANCE FROM CENTER OF PUMPED WELL	ZP	DELAYED RESPONSE FACTOR
0.507D+01	0.890D+00	0.173D+05

DRAWDOWN CALCULATED FOR BETA = 0.284D+00

TIME	CALCULATED DRAWDOWN
0.2240D+00	0.4495D-03
0.3360D+00	0.1374D-02
0.4853D+00	0.3359D-02
0.6720D+00	0.6745D-02
0.1006D+01	0.1406D-01
0.1527D+01	0.2537D-01
0.2227D+01	0.3695D-01
0.3276D+01	0.4659D-01
0.4844D+01	0.5228D-01
0.6796D+01	0.5509D-01
0.1011D+02	0.5810D-01
0.1507D+02	0.6191D-01
0.2250D+02	0.6699D-01
0.3173D+02	0.7270D-01
0.4742D+02	0.8145D-01
0.6694D+02	0.9123D-01
0.1001D+03	0.1060D+00
0.1497D+03	0.1250D+00
0.2240D+03	0.1491D+00
0.3163D+03	0.1739D+00
0.4733D+03	0.2076D+00
0.6684D+03	0.2401D+00
0.1000D+04	0.2814D+00
0.1496D+04	0.3254D+00
0.2239D+04	0.3711D+00
0.3162D+04	0.4111D+00

0.4697D+04 0.4575D+00
 0.6857D+04 0.5024D+00
 0.1046D+05 0.5527D+00

OBSERVATION WELL OR PIEZOMETER: WD4A

OBSERVATION PIEZOMETER

DISTANCE FROM CENTER OF PUMPED WELL	ZP	DELAYED RESPONSE FACTOR
0.151D+02	0.840D+00	0.173D+05

DRAWDOWN CALCULATED FOR BETA = 0.250D+01

TIME	CALCULATED DRAWDOWN
0.4875D+00	0.1158D-03
0.6750D+00	0.3477D-03
0.1025D+01	0.1075D-02
0.1493D+01	0.2299D-02
0.2193D+01	0.3944D-02
0.3241D+01	0.5468D-02
0.4810D+01	0.6435D-02
0.6762D+01	0.6940D-02
0.1008D+02	0.7499D-02
0.1504D+02	0.8223D-02
0.2246D+02	0.9220D-02
0.3170D+02	0.1039D-01
0.4739D+02	0.1227D-01
0.6691D+02	0.1452D-01
0.1001D+03	0.1822D-01
0.1497D+03	0.2359D-01
0.2239D+03	0.3135D-01
0.3163D+03	0.4061D-01
0.4732D+03	0.5540D-01
0.6684D+03	0.7217D-01
0.1000D+04	0.9700D-01
0.1496D+04	0.1274D+00
0.2239D+04	0.1628D+00
0.3162D+04	0.1963D+00
0.4697D+04	0.2373D+00
0.6857D+04	0.2785D+00
0.1046D+05	0.3260D+00

OBSERVATION WELL OR PIEZOMETER: P17

OBSERVATION PIEZOMETER

DISTANCE FROM CENTER OF PUMPED WELL	ZP	DELAYED RESPONSE FACTOR
0.515D+01	0.269D+01	0.173D+05

DRAWDOWN CALCULATED FOR BETA = 0.293D+00

TIME	CALCULATED DRAWDOWN
0.3750D-01	0.1620D-05
0.7500D-01	0.3526D-04
0.1125D+00	0.1576D-03
0.1500D+00	0.4106D-03
0.2250D+00	0.1395D-02
0.3375D+00	0.4145D-02
0.4875D+00	0.9952D-02
0.6750D+00	0.1972D-01
0.1025D+01	0.4146D-01
0.1493D+01	0.6992D-01
0.2193D+01	0.1021D+00
0.3241D+01	0.1276D+00
0.4810D+01	0.1404D+00
0.6762D+01	0.1444D+00
0.1008D+02	0.1469D+00
0.1504D+02	0.1497D+00
0.2246D+02	0.1535D+00
0.3170D+02	0.1578D+00
0.4739D+02	0.1643D+00
0.6691D+02	0.1717D+00
0.1001D+03	0.1829D+00
0.1497D+03	0.1976D+00
0.2239D+03	0.2165D+00
0.3163D+03	0.2363D+00
0.4732D+03	0.2640D+00
0.6684D+03	0.2915D+00
0.1000D+04	0.3275D+00
0.1496D+04	0.3671D+00
0.2239D+04	0.4093D+00
0.3162D+04	0.4471D+00
0.4697D+04	0.4916D+00
0.6857D+04	0.5351D+00
0.1046D+05	0.5845D+00

OBSERVATION WELL OR PIEZOMETER: P4

OBSERVATION PIEZOMETER

DISTANCE FROM CENTER OF PUMPED WELL	ZP	DELAYED RESPONSE FACTOR
0.154D+02	0.232D+01	0.943D+04

DRAWDOWN CALCULATED FOR BETA = 0.260D+01

TIME	CALCULATED DRAWDOWN
0.6750D+00	0.1295D-02
0.1025D+01	0.3706D-02
0.1493D+01	0.7187D-02
0.2193D+01	0.1100D-01
0.3241D+01	0.1373D-01
0.4810D+01	0.1505D-01
0.6762D+01	0.1562D-01
0.1008D+02	0.1621D-01
0.1504D+02	0.1699D-01
0.2246D+02	0.1804D-01
0.3170D+02	0.1927D-01
0.4739D+02	0.2124D-01
0.6691D+02	0.2356D-01
0.1001D+03	0.2732D-01
0.1497D+03	0.3268D-01
0.2239D+03	0.4029D-01
0.3163D+03	0.4922D-01
0.4732D+03	0.6328D-01
0.6684D+03	0.7907D-01
0.1000D+04	0.1024D+00
0.1496D+04	0.1311D+00
0.2239D+04	0.1646D+00
0.3162D+04	0.1967D+00
0.4697D+04	0.2364D+00
0.6857D+04	0.2766D+00
0.1046D+05	0.3233D+00

OBSERVATION WELL OR PIEZOMETER: P14

OBSERVATION PIEZOMETER

DISTANCE FROM CENTER OF PUMPED WELL	ZP	DELAYED RESPONSE FACTOR
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40 WTAQ Version 2—A Computer Program for Analysis of Aquifer Tests in Confined and Water-Table Aquifers

0.151D+01 0.457D+01 0.173D+05

DRAWDOWN CALCULATED FOR BETA = 0.252D-01

TIME	CALCULATED DRAWDOWN
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0.1493D+00	0.6028D-02
0.2240D+00	0.1464D-01
0.3360D+00	0.3357D-01
0.4853D+00	0.6728D-01
0.6720D+00	0.1178D+00
0.1006D+01	0.2156D+00
0.1527D+01	0.3534D+00
0.2227D+01	0.4835D+00
0.3276D+01	0.5816D+00
0.4844D+01	0.6271D+00
0.6796D+01	0.6381D+00
0.1011D+02	0.6415D+00
0.1507D+02	0.6443D+00
0.2250D+02	0.6480D+00
0.3173D+02	0.6522D+00
0.4742D+02	0.6585D+00
0.6694D+02	0.6656D+00
0.1001D+03	0.6765D+00
0.1497D+03	0.6906D+00
0.2240D+03	0.7087D+00
0.3163D+03	0.7277D+00
0.4733D+03	0.7544D+00
0.6684D+03	0.7809D+00
0.1000D+04	0.8158D+00
0.1496D+04	0.8543D+00
0.2239D+04	0.8957D+00
0.3162D+04	0.9330D+00
0.4697D+04	0.9770D+00
0.6857D+04	0.1020D+01
0.1046D+05	0.1069D+01

OBSERVATION WELL OR PIEZOMETER: P5

OBSERVATION PIEZOMETER

DISTANCE FROM CENTER OF PUMPED WELL	ZP	DELAYED RESPONSE FACTOR
0.302D+02	0.430D+01	0.000D+00

DRAWDOWN CALCULATED FOR BETA = 0.101D+02

TIME	CALCULATED DRAWDOWN
0.4800D+03	0.1167D-01
0.1080D+04	0.2662D-01
0.1950D+04	0.4809D-01
0.3420D+04	0.7954D-01
0.4920D+04	0.1057D+00
0.6750D+04	0.1318D+00
0.1053D+05	0.1728D+00

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