



CONSERVATION OF MASS
(OR CONTINUITY EQ'N)
RELATIONS

FIGURE 2.3.1 Cross-section of an unconfined aquifer

Non-steady state:
 $H = f(x, y, z, t)$
 Steady state:
 $H = g(x, y, z)$

Hydraulic Approach to groundwater flow:

- a) Most aquifers are longer (km's) than thicker (10's of meters)
- b) Law of Refraction of flow lines suggest flow is mostly horizontal. Thus, on a large scale "flow in aquifers is mostly horizontal and vertical in confining beds."
 - ∴ Velocity has 2 main components, Q_x & Q_y (w) $Q_z = 0$
 - ∴ Head varies only w/ horizontal location and time or $h = f(x, y, t)$
 - except at partially-penetrating wells, by rivers, among others, but it meets it $\sim (1.5-2.0) \times$ the aquifer thickness

The Dupuit-Forchheimer Assumptions (for an unconfined aquifer)

- 1) $h(x, y, z, t) \neq f(z) \therefore h(x, y, t) \rightarrow$ Mostly horizontal Q
- 2) Discharge $Q \propto$ slope of water table or $h(x, y, t)$
- 3) ρ & n = constant in unconfined aquifers
- 4) Fluxes or q_s are q_x or $U_x = -K_x (H - \xi) \frac{\partial H}{\partial x}$
 q_y or $U_y = -K_y (H - \xi) \frac{\partial H}{\partial y}$

Example of Derivation of Equation (and applications)

From the continuity equation:

$$\text{rate of volume accumulation (or storage)} = \text{net volume flux out} - \text{rate volume flux in} \pm \text{rate volume sources or sinks}$$

From Figure 2.3.1 between x & $x + \Delta x$, using Darcy's Law:

$$S_y \frac{\partial H}{\partial t} = \frac{\partial}{\partial x} \left[K(H - \xi) \right] \frac{\partial H}{\partial x} + W \quad (2.3.9)$$

2nd order Partial D.E. (or PDE)

that should be solved for Initial and Boundary conditions

- a) via analytical solutions for simplified settings and
- b) numerical solutions (or "matrix algebra")

Some possible simplifications are:

If flow is steady: $\sum y \frac{\partial H}{\partial t} = 0$

$$\therefore K \frac{d}{dx} \left(H \frac{dH}{dx} \right) + W = 0 \quad (\text{if } \zeta = 0)$$

∴ 2nd order Ordinary D.E. or ODE

If 1-D situations (Eq. 2.3.9).

$$H \frac{dH}{dx} = \frac{1}{2} \frac{dH^2}{dx}$$

$$\therefore \frac{d}{dx} \left(\frac{dH^2}{dx} \right) + \frac{2W}{K} = 0$$

If $W = \text{constant}$, after integration,

$$\therefore H^2 + \frac{Wx^2}{K} = Ax + B, \quad A \text{ \& } B = \text{constants of integration.}$$

If 2-D situations (Eq. 2.3.16)

$$\nabla^2(H^2) + 2W/K = 0 \quad (2.3.17)$$

(Poisson's Eq.)

$$\frac{\partial^2(H^2)}{\partial x^2} + \frac{\partial^2(H^2)}{\partial y^2} + \frac{\partial^2(H^2)}{\partial z^2}$$

→ Laplacian Operator (Appendix A, p. 534)

whose solution in 1-D for radial coordinates yields

$$\frac{1}{r} \frac{d}{dr} \left(r \frac{dH^2}{dr} \right) + \frac{2W}{K} = 0$$

$$\therefore H^2 + \frac{Wr^2}{2K} = A \ln(r) + B \quad (2.3.18)$$

or Dupuit's Eq.

(for unconfined aquifers)